

STATE OF HAWAII  
DEPARTMENT OF TRANSPORTATION  
HIGHWAYS

ADDENDUM NO. 1

FOR

KAILUA ROAD  
TRAFFIC INTERSECTION IMPROVEMENTS ON KAILUA ROAD,  
VICINITY OF ULUOA STREET AND ULUMANU DRIVE  
DISTRICTS OF KOOLAUPOKO  
ISLAND OF OAHU  
PROJECT NO. 61D-01-23

January 11, 2024

This Addendum shall make the following amendments to the Bid Documents:

**A. SOLICITATION OFFER DUE DATE & TIME**

1. The Solicitation Offer Due Date & Time is hereby corrected from "01/18/2024 12:00 AM" to "**01/18/2024 2:00 PM**".

**B. NOTICE TO BIDDERS**

1. Delete **NOTICE TO BIDDERS** in its entirety and replace with attached **NOTICE TO BIDDERS** dated r1/11/2024.

**C. SPECIAL PROVISIONS**

1. Delete **SECTION 623 – TRAFFIC SIGNAL SYSTEM** dated 10/03/2023 in its entirety and replace with attached **SECTION 623 – TRAFFIC SIGNAL SYSTEM** dated r1/11/2024.
2. Delete **SECTION 627 – TRAFFIC MONITORING AND SIGNAL CONTROL SYSTEM** dated 12/06/2023 in its entirety and replace with attached **SECTION 627 – TRAFFIC MONITORING AND SIGNAL CONTROL SYSTEM** dated r1/11/2024.
3. Add and make a part of the Specifications the attached **SECTION 647 – FIBER OPTION CABLE** dated r1/11/2024.

#### **D. PROPOSAL SCHEDULE**

1. Delete **PROPOSAL SCHEDULE** dated 12/13/2023 in its entirety and replace with attached **PROPOSAL SCHEDULE** dated 1/11/2024.

#### **E. PLANS:**

1. Delete Plan Sheet No. **1** – Title Sheet and replace with attached Plan Sheet No. **ADD. 1** – TITLE SHEET.
2. Delete Plan Sheet No. **4** – General Notes & Legend – 3 and replace with attached Plan Sheet No. **ADD. 4** – General Notes & Legend – 3.
3. Delete Plan Sheet No. **27** – Curb Ramp Details – 2 and replace with the attached Plan Sheet No. **ADD. 27** – Curb Ramp Details – 2.
4. Delete Plan Sheet No. **30** – Curb Ramp Details – 5 and replace with the attached Plan Sheet No. **ADD. 30** – Curb Ramp Details – 5.
5. Delete Plan Sheet No. **33** – Curb Ramp Details – 8 and replace with the attached Plan Sheet No. **ADD. 33** – Curb Ramp Details – 8.
6. Delete Plan Sheet No. **39** – Paving Notes & Details and replace with the attached Plan Sheet No. **ADD. 39** – Paving Notes & Details.
7. Delete Plan Sheet No. **40** – Paving Plan and replace with the attached Plan Sheet No. **ADD. 40** – Paving Plan.
8. Delete Plan Sheet No. **66** – Drilled Shaft Foundation Details and replace with the attached Plan Sheet No. **ADD. 66** – Drilled Shaft Foundation Details.
9. Add and make a part of the Plans the attached Plan Sheet No. **ADD. 66S-1**.
10. Delete Plan Sheet No. **67** – Boring Log – 1 and replace with the attached Plan Sheet No. **ADD. 67** – Boring Log – 1.
11. Delete Plan Sheet No. **68** – Boring Log – 2 and replace with the attached Plan Sheet No. **ADD. 68** – Boring Log – 2.
12. Delete Plan Sheet No. **81** – Ulua Power and Street Light Plan and replace with the attached Plan Sheet No. **ADD. 81** – Ulua Power and Street Light Plan.

The following is provided for information.

**F. PRE-BID MEETING MINUTES**

1. The attached PRE-BID MEETING MINUTES and ATTENDANCE SHEET dated December 28, 2023, is provided for information.

**G. GEOTECHNICAL ENGINEERING REPORT FOR KAILUA ROAD INTERSECTION IMPROVEMENTS**

1. The attached GEOTECHNICAL ENGINEERING REPORT FOR KAILUA ROAD INTERSECTION IMPROVEMENTS, dated December 22, 2023, is provided for information.

**H. RESPONSES TO REQUEST FOR INFORMATION (RFI'S/QUESTIONS)**

1. The attached Responses to Request for Information is provided for information.

Please acknowledge receipt of this Addendum No. 1 by recording the date of its receipt in the space provided on Page P-4 of the Proposal.



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ROBIN K. SHISHIDO  
Deputy Director of Transportation for Highways

**NOTICE TO BIDDERS**  
Hawaii Revised Statutes (HRS),  
Chapter 103D

SEALED BIDS for KAILUA ROAD, TRAFFIC INTERSECTION IMPROVEMENTS  
ON KAILUA ROAD, VICINITY OF ULUOA STREET AND ULUMANU DRIVE, DISTRICT  
OF KOOLAUPOKO, ISLAND OF OAHU, PROJECT NO. 61D-01-23, will begin as advertised  
in HiePRO. Bidders shall register and submit complete bids through HiePRO only. Refer to the  
following HiePRO link for important information on registering:

<https://hiepro.ehawaii.gov/welcome.html>.

Plans, specifications, proposal, and other documents designated or incorporated by  
reference shall be available in HiePRO.

DEADLINE TO SUBMIT BIDS is January 18, 2024, at 2:00 p.m., Hawaii Standard  
Time (HST). **Bidders shall submit and upload the complete proposal to HiePRO prior to  
the bid opening date and time. Proposals received after said due date and time shall not be  
considered. Any additional support documents explicitly designated as confidential and/or  
proprietary shall be uploaded as a separate file to HiePRO. Do not include confidential  
and/or proprietary documents with the proposal. The record of each bidder and respective  
bid shall be open to public inspection. FAILURE TO UPLOAD THE PROPOSAL TO  
HiePRO SHALL BE GROUNDS FOR REJECTION OF THE BID.**

The scope of work consists of signaling the intersections of Kailua Road and  
Uluoa Street, and Kailua Road and Ulumanu Drive, including but not limited to the installation  
of Type II traffic signal poles, Type I traffic signal poles, interconnect ducts, pedestrian push  
button assemblies, traffic signal boxes, and traffic loops along Kailua Road from  
Waimanlo Junction to Ulumanu Drive; construction of sidewalk curb extensions or bulb-outs,



sidewalk curb ramps, sidewalk and pavement reconstruction; striping and signage; and work zone traffic control. The estimated cost of construction is between \$5,000,000 and \$5,500,000.

To be eligible for award, bidders shall possess a valid State of Hawaii General Engineering "A" license at the time of bidding. Bidder's attention is also directed to Section 627.01 of the Special Provisions regarding additional bidder's qualification.

Due to the intricate nature of HDOT's CCTV and fiber optic cable system, the Bidder's CCTV supplier and fiber optic cable installer shall have previous experience working with the City and County of Honolulu or HDOT's existing CCTV system for at least three (3) previous projects. Written documentation confirming previous experience working on either CCTV systems or fiber systems shall be submitted to the Project Manager before January 25, 2024, at 3:30 PM (within five (5) working days after bid opening).

A pre-bid conference is scheduled for Pre-bid meeting December 28, 2023, at 2:00 p.m., HST, on Microsoft Teams. Due to the impacts of COVID 19, the pre-bid meeting will be conducted virtually. Contact Mr. Reid Tokuhara, Project Manager, at (808) 692-7691, or by email at reid.tokuhara@hawaii.gov, at minimum of 48-hours prior to the scheduled pre-bid meeting to obtain the link for the pre-bid meeting. All prospective bidders and/or their respective representatives are encouraged to attend, however, attendance is not mandatory. All information presented at the pre-bid conference is provided for clarification and information only. Any amendments to the bid documents shall be made by formal addendum and posted in HIePRO.

All Request for Information (RFI) questions and substitution requests shall be submitted via HIePRO **no later than January 2, 2024, at 2:00 p.m., HST**. RFI questions received after the stated deadline will not be addressed. Verbal RFI questions will not receive a response. All responses to RFI questions shall be issued by formal addendum and posted in HIePRO.

Apprenticeship Preference. A 5% bid adjustment for bidders that are party to apprenticeship agreements pursuant to HRS §103-55.6 is applicable to this project.

Employment of State Residents on Construction Procurement Contracts. Compliance with HRS §103B-3 is a requirement for this project whereby a minimum of 80% of the bidder's work force on this project shall consist of Hawaii residents.

Campaign contributions by State and County Contractors. Contractors are hereby notified of the applicability of HRS §11-355 which states that campaign contributions are prohibited from specified State or county government contractors during the term of the contract if the contractors are paid with funds appropriated by a legislative body. For more information, contact the Campaign Spending Commission at (808) 586-0285.

Protests. Any protest of this solicitation shall be submitted in writing to the Director of Transportation, in accordance with HRS §103D-701 and Hawaii Administrative Rules §3-126.

The Equal Employment Opportunity Regulations of the Secretary of Labor implementing Executive Order 11246, as amended, shall be complied with on this project.

The U.S. Department of Transportation Regulation entitled “Nondiscrimination in Federally-Assisted Programs of the U.S. Department of Transportation,” Title 49, Code of Federal Regulations (CFR), Part 21 is applicable to this project. Bidders are hereby notified that the Department of Transportation will affirmatively ensure that the contract entered into pursuant to this advertisement will be awarded to the lowest responsible bidder without discrimination on the grounds of race, color, national origin or sex (as directed by 23 CFR Part 200).

Driving While Impaired (DWI) Education. HDOT encourages all organizations contracted with the DOT to have an employee education program preventing DWI. DWI is defined as operating a motor vehicle while impaired by alcohol or other legal or illegal

substances. HDOT promotes this type of program to accomplish our mission to provide a safe environment for motorists, bicyclists and pedestrians utilizing our State highways, and expects its contractors to do so as well.

For additional information, contact Reid Tokuhara, Project Manager, by phone at (808) 692-7691, by fax at (808) 692-7690 or email at reid.tokuhara@hawaii.gov address.

The State reserves the right to reject any or all proposals and to waive any defects in said proposals in the best interest of the public.



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ROBIN K. SHISHIDO  
Deputy Director of Transportation for Highways

Posted on HiePRO: January 11, 2024



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- (3) Detector for Traffic Actuation** - Device that pedestrians or vehicles can register their presence with traffic-actuated controller.
  - (4) Extendible Portion** - That part of green interval that follows initial portion.
  - (5) Extension Limit** - Maximum time that traffic phase may retain right-of-way after actuation on another traffic phase, after timing out initial portion.
  - (6) Flashing Feature** - Feature incorporated to stop normal signal operation and cause flashing of predetermined combination of signal lights.
  - (7) Initial Portion** - Part of green interval that is timed-out or separately controlled by traffic-actuated controller before extendible portion of interval takes effect.
  - (8) Interval** - Several divisions of time cycle during which signal indications do not change.
  - (9) Interval Sequence** - Order of appearance of signal indications during successive intervals of time cycle.
  - (10) Magnetic Vehicle Detector** - Detector actuated by movement of vehicle passing through magnetic field.
  - (11) Major Street** - Roadway approach or approaches at intersection normally carrying greater volume of vehicular traffic.
  - (12) Manual Operation** - Operation of signal controller by hand-operated switch.
  - (13) Minimum Period** - In semi-traffic-actuated controllers, shortest time for which right-of-way will be given to approaches not having detectors.
  - (14) Minor Movement Interval** - Auxiliary phase added to controller phase (parent phase) and modified by auxiliary movement controller.
  - (15) Minor Street** - Roadway approach or approaches at intersection normally carrying smaller volume of vehicular traffic.
  - (16) Non-Parent Phase** - Controller phase not modified by auxiliary control unit.

- 95 (17) **Parent Phase** - Controller phase modified by auxiliary control unit.  
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97 (18) **Passage Period** - Time allowed for vehicle to travel at selected  
98 speed from detector to nearest point of conflicting traffic.  
99  
100 (19) **Pedestrian Detector** - Detector, usually of push-button type,  
101 installed near roadway and operated by hand.  
102  
103 (20) **Pressure-Sensitive Vehicle Detector** - Detector installed in  
104 roadway, actuated by pressure of vehicle passing over its surface.  
105  
106 (21) **Pre-Timed Controller** - Automatic control device for supervising  
107 operation of traffic control signals in accordance with pre-timed cycle and  
108 divisions.  
109  
110 (22) **Recall Switch** - Manually operated switch in actuated controller to  
111 provide for automatic return of right-of-way to street.  
112  
113 (23) **Right-of-Way** - Privilege of immediate use of highway.  
114  
115 (24) **Signal Indication** - Illumination of traffic signal lens or equivalent  
116 device, or of combination of several lenses or equivalent devices.  
117  
118 (25) **Time Cycle** - Number of seconds required for one complete  
119 revolution of timing dial or complete sequence of signal indications.  
120  
121 (26) **Traffic-Actuated Controller** - Digital control device for supervising  
122 operation of traffic control signals in accordance with varying demands of  
123 traffic as registered with controller by loop detectors or pedestrian push  
124 buttons.  
125  
126 (27) **Traffic Phase** - Part of cycle allocated to traffic movements  
127 receiving right-of-way or to combinations of traffic movements receiving  
128 right-of-way simultaneously during one or more intervals.  
129  
130 (28) **Unit Extension** - Minimum time, during extendible portion, for  
131 which right-of-way must remain on traffic phases following actuation on  
132 that phase, subject to extension limit”  
133

134 (III) Amend **623.02 Materials** by adding the following after line 132:

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136 “Pedestrian Signal Push Button with Integral Sign 770.12”  
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138 (IV) Amend **Subsection 623.03(C)(7)** from lines 255 to 258 to read as follows:

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**“(7) Conduits.** Lay polyvinyl chloride (PVC) conduits carefully in trenches prepared to receive conduits. Concrete encase PVC Schedule 40 conduits.”

**(V) Amend Section 623.04 Measurement and 623.05 Payment** from lines 578 to 594 to read as follows:

**“623.04 Measurement.** The Engineer will not measure firmware for controller, for payment.

(A) The Engineer will measure the controller assembly, foundation for traffic signal controller, traffic signal standard, foundation for traffic signal standard, pedestrian or traffic signal assembly, pedestrian pushbutton, pullbox, loop detector sensing unit, and emergency vehicle preemption receiver per each in accordance with the contract documents.

(B) The Engineer will measure traffic signal ductline, conductors, and EVP cable per linear foot in accordance with the contract documents.

**623.05 Payment.** The Engineer will pay for the accepted controller assembly at the contract unit price per each complete in place. The price includes full compensation for submitting the equipment list and drawing; furnishing and mounting the controller cabinet; furnishing, assembling, wiring, firmware, and housing the controller and auxiliary equipment; painting the controller cabinet; testing; providing turn-on service; submitting warranty; and furnishing equipments, tools, labor, materials and other incidentals necessary to complete the work.

The Engineer will pay for the accepted traffic signal standard at the contract unit price per each complete in place. The price includes full compensation for submitting the equipment list and drawing; furnishing and installing the traffic signal standard; wiring; bonding and grounding; testing; providing turn-on service; submitting warranty; and furnishing equipments, tools, labor, materials, and other incidentals necessary to complete the work.

The Engineer will pay for the accepted foundation for controller cabinet and traffic signal standard at the contract unit price per each complete in place. The price includes full compensation for excavating and backfilling; forming; furnishing and placing the reinforcing steel; mixing, placing, and curing the concrete; furnishing and setting the anchor bolts; restoring the pavement; and furnishing equipments, tools, labor, materials and other incidentals necessary to complete the work.

The Engineer will pay for the accepted pedestrian and traffic signal assembly at the contract unit price per each complete in place. The price

185 includes full compensation for submitting the equipment list and drawing;  
186 assembling the signal heads; wiring; bonding and grounding; painting the signal  
187 head mounting; testing; providing turn-on service; submitting warranty; and  
188 furnishing equipments, tools, labor, materials and other incidentals necessary to  
189 complete the work.

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191 The Engineer will pay for the accepted emergency vehicle preemption  
192 (EVP) optical receiver at the contract unit price per each complete in place. The  
193 price includes full compensation for submitting the equipment list and drawing;  
194 assembling the EVP; wiring; bonding and grounding; testing; providing turn-on  
195 service; submitting warranty; and furnishing equipments, tools, labor, materials  
196 and other incidentals necessary to complete the work.

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198 The Engineer will pay for the accepted pedestrian piezo electric  
199 pushbutton with instruction sign at the contract unit price per each complete in  
200 place. The price includes full compensation for submitting the equipment list and  
201 drawing; furnishing and installing the pedestrian pushbutton with the instruction  
202 sign; wiring; bonding and grounding; testing; providing turn-on service; submitting  
203 warranty; and furnishing equipments, tools, labor, materials and other incidentals  
204 necessary to complete the work.

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206 The Engineer will pay for the accepted traffic signal ductline at the  
207 contract unit price per linear foot complete in place. The price includes full  
208 compensation for saw cutting; trenching; excavating and backfilling, including  
209 asphalt concrete pavement, aggregate base course and aggregate subbase  
210 course for trench repair; concrete curb and/or gutter and concrete sidewalk  
211 repair; furnishing, installing, bonding, and grounding the conduits and  
212 interconnect subducts; and furnishing equipments, tools, labor, materials and  
213 other incidentals necessary to complete the work.

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215 The Engineer will pay for the accepted pullbox at the contract unit price  
216 per each complete in place. The price includes full compensation for submitting  
217 the equipment list and drawing; furnishing and installing the pullbox at the  
218 designated locations; saw cutting; excavating and backfilling; restoration of  
219 concrete sidewalks, asphalt concrete pavement and landscaping; coating the  
220 frames and covers; and furnishing equipments, tools, labor, materials and other  
221 incidentals necessary to complete the work.

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223 The Engineer will pay for the accepted traffic signal and EVP cables at  
224 the contract unit price per linear foot complete in place. The price includes full  
225 compensation for furnishing, installing, splicing, and taping the cable; making the  
226 connections; providing turn-on service; and furnishing equipments, tools, labor,  
227 materials and other incidentals necessary to complete the work.

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229 The Engineer will pay for the accepted loop detector sensing unit at the  
230 contract unit price per each complete in place. The price includes full



231 compensation for saw cutting; cleaning and blowing the saw cut area; furnishing  
232 and inserting the loop cable; splicing in the pullbox; filling the saw cut groove with  
233 epoxy sealer or hot applied rubberized sealant; and furnishing equipments, tools,  
234 labor, materials and other incidentals necessary to complete the work.

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236 The Engineer will consider full compensation for additional materials  
237 and labor not specifically shown or called for that are necessary to complete the  
238 work incidental to the various contract items in the proposal.

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240 The Engineer will pay for each of the following pay items when  
241 included in the proposal schedule:

242	<b>Pay Item</b>	<b>Pay Unit</b>
243		
244	_____ Controller Assembly with Firmware _____	Each
245		
246	Type _____ Traffic Signal Standard _____	Each
247		
248	Foundation for _____	Each
249		
250	_____ Signal Assembly _____	Each
251		
252	EVP Optical Receiver with _____	Each
253		
254	Pedestrian Pushbutton with Instruction Sign	Each
255		
256	Traffic Signal Ductline _____	Lin. Ft.
257		
258	_____ Type _____ Pullbox	Each
259		
260	No. _____, _____ Cable	Lin. Ft.
261		
262	EVP Cable	Lin. Ft.
263		
264	Loop Detector Sensing Unit (6 Ft. x 6 Ft.) _____ Loops	Each
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266 Payment shall be full compensation for the work prescribed in this  
267 section and the contract documents. The Engineer shall consider additional  
268 materials and labor not specifically shown or called for that are necessary to  
269 complete the work as incidental to the various contract items in the proposal  
270 schedule.”

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**END OF SECTION 623**

1 Make the following section part of the Standard Specifications:  
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3 **"SECTION 627 – TRAFFIC MONITORING AND SIGNAL CONTROL SYSTEM**  
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5 **627.01 DESCRIPTION.** This section shall consist of all work and materials necessary  
6 to complete a fully operational CCTV and signal control system for traffic control and  
7 surveillance of various sites shown on the plans. The work shall involve coordinating all  
8 equipment and labor necessary to incorporate and integrate the two new signalized  
9 intersections into HDOT's H-3 Traffic Operations Center (TOC) and/or City's Joint  
10 Traffic Management Center (JTMC) systems, using Internet Protocol (IP) based  
11 communications. The expanded CCTV and signal control system will assist operators  
12 at the TOC and/or JTMC to monitor traffic conditions, mitigate traffic congestion, and set  
13 the appropriate traffic plans which best suits and improves the traffic progression along  
14 Oahu's busiest arterials.  
15

16 The CCTV and signal control system shall consist of remotely controlled color cameras,  
17 remote video switching, IP communications system, cellular modems and a fiber optic  
18 interconnect system. The local traffic signal control system will send control data  
19 transmitted over two single-mode fibers through a 100/1000/10000base T/FX IP switch.  
20 At the Uluoa intersection a cellular modem will transmit the control data to a traffic  
21 signal central server located at the JTMC. In addition, the traffic surveillance CCTV  
22 cameras will be connected directly to a cellular modem and video data will be  
23 transmitted to the TOC servers.  
24

25 All CCTV camera equipment shall be identical and/or compatible with the City's and  
26 HDOT's existing CCTV system in terms of hardware and software.  
27

28 Due to the intricate nature of HDOT's CCTV and fiber optic cable system, the Bidder's  
29 CCTV supplier shall have previous experience working with the City and County of  
30 Honolulu or HDOT's existing CCTV system for at least three (3) previous projects.  
31 Written documentation confirming previous experience working on either CCTV systems  
32 or fiber systems shall be submitted to the project Manager before January 25, 2024, at  
33 3:30 PM (within five (5) working days after bid opening).  
34

35 The CCTV firm shall be responsible for testing all fiberoptic hardware and cables to  
36 provide a documented optical budget loss analysis for each link to and from a hub  
37 station. The CCTV supplier will be responsible for all hookup, assignments, dedication,  
38 testing, matching, and splicing of the fiberoptic cables. All fiberoptic splice points shall  
39 have pigtails on all fiberoptic members which attach to fiberoptic hardware and  
40 components with SC-connectors. Six strands of the same buffer tube shall be jumpered  
41 color for color using a SC-connectors fiber optic patch panel. Patch cords shall be  
42 provided for the six strands connected to the patch panel. All remaining fiber optic  
43 strands shall be fusion spliced color for color. The CCTV supplier shall be fully  
44 responsible for all splices, budget loss, attenuators, appropriate fiber hardware,  
45 accessories, and pigtail connections for a fully operational system. All other hardware,  
46 equipment, and labor necessary shall be considered incidental.

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The firm shall track and document the installation data and tension measurements when installing the fiberoptic cables. Any tension measurements which exceeds the manufacturer's recommendations will be considered means for the cable rejection. The Fiberoptic Contractor shall be fully responsible for the quality and integrity of the installed cable and the operability of the final fiberoptic cable product. The Fiberoptic Cable Contractor shall be responsible for testing all fiber optic strands and to provide a documented optical budget loss analysis report showing the acceptable budget losses from one end to the other end of all fiber optic strands.

**627.02 TRAFFIC SIGNAL CONTROL SYSTEM.** For bidding purposes, the CCTV Supplier shall furnish and install all the necessary items to provide traffic signal control from the JTMC, to all three traffic signal controllers, utilizing HDOT's existing central server. All other equipment necessary to complete a fully operational system will be considered incidental.

The traffic signal controller will communicate with the JTMC over an Ethernet network.

The Contractor shall at each new signalized intersection furnish and install, but not limited to, the following items:

**(A) Traffic Signal Central Server.** The Contractor shall furnish and install the necessary licenses that will allow the two new signalized intersections to communicate and work with HDOT's traffic signal central server.

**(B) CCTV Cabinet.** A CCTV cabinet with foundation shall be provided at each new signalized intersection. All cabinet shall be furnished assembled and configured with the components stated below:

Cabinet shall be a Traffic Signal 332LS anodized aluminum cabinet with a 19" rack, 50 amp circuit breaker, surge-protected, and thermo-control fan.

Each Model 332LS Cabinet shall meet the following additional requirements:

- (1)** Provide Best Lock (C&C of Honolulu keyed) Security Tumbler Door locks of solid brass rim and include 4 keys.
- (2)** A rack mounted 6 outlet surge protector power strip
- (3)** A 19 inch pull out shelf
- (4)** Remote data port with monitor and control, Stand Alone, all connectors and cables included
- (5)** Rack Mounted 72 fiber optic Splice Capacity Tray
- (6)** Rack Mounted 72 fiber optic SC jumper connector

Surge Protection: Contractor shall install a 120V AC, 3-wire, 20 Amp inline surge protection device. The surge protection device will have an operating temperature of -40 to 85 degree C, maximum surge current of 30,000 amps

93 and surge voltage of 10,000 volts, 138 Volts for clamping voltage, power  
94 indicator, open circuit for fail safe operation, and protection shall be between  
95 line to neutral, line to ground, ground to neutral.  
96

97 Furnish and install power cables from existing traffic signal meter or new Hawaiian  
98 Electric service point.  
99

100 **(C) Hardened Ethernet Switch.** The network managed Layer 2, with light Layer 3  
101 managed switch is a hardened DIN-rail mounted managed PoE++ Ethernet  
102 switch equipped with 12 gigabit PoE++ ports along with 360W power and IEEE  
103 802.3bt protocol support and 4 dual rate 1G/10G SFP ports. The managed  
104 switch shall be optically and electrically compatible with any IEEE 802.3  
105 compliant Ethernet devices. The managed Ethernet switch will provide  
106 transmission of eight 100/1000 BASE-TX and four 1/10G FX ports. The  
107 managed Ethernet switches shall be environmentally hardened units, designed  
108 for roadside operating environments, and are available for use with either  
109 conventional CAT 6 copper or optical transmission media. CAT 6 cables shall  
110 be provided between the switch and the traffic signal controllers. The twelve  
111 electrical ports support the 10/100/1000 Mbps Ethernet IEEE 802.3 protocol,  
112 auto-negotiating, and auto-MDI/MDIX, four 1/10G FX ports are configurable for  
113 copper or fiber media for use with multimode or single mode optical fiber,  
114 selected by optional SFP modules, plug-and-play design, and no electrical or  
115 optical adjustments required. LED indicators for monitoring the operating  
116 status of the managed switch and network and is either DIN-rail or wall  
117 mountable.  
118

119 The hardened managed Ethernet switch shall meet the following minimum  
120 requirements:  
121

- 122 **(1)** Layer 2 with light Layer 3 managed switch
- 123 **(2)** Layer 3 Features at a minimum includes IP Packet Routing (64  
124 hardware routes, Static routing, RIP v1/v2, OSPF v2) and Routing  
125 Redundancy
- 126 **(3)** Transmission of 4 channels of 1/10G over one or two single-mode  
127 fibers respectively.
- 128 **(4)** Transmission of 12 channels of 10/100/1000 Mbps over Cat-6 cable.
- 129 **(5)** 2 – Hardened Single (LC), 1 Gigabit, 40 Km SFP modules.
- 130 **(6)** 2 – Hardened Duplex (LC), 1 Gigabit, 40 Km SFP modules.
- 131 **(7)** 2 – Hardened Duplex (LC), 10 Gigabit, 40 Km SFP modules (1310  
132 nm).
- 133 **(8)** Up to 90W per PoE port, with a power budget of 360 Watts.  
134 Compliance to IEEE 802.3bt type 4.
- 135 **(9)** Shall support the Ethernet data IEEE 802.3 protocol using Auto-  
136 negotiating for port speed and duplex.
- 137 **(10)** Provide power, link speed, and fiber port status indicating LED's for  
138 monitoring system operation.

- 139 (11) Provide 2 - alarm contact closure.  
140 (12) Power Supply: 480W / 10A DIN Rail, 48VDC Industrial Power Supply,  
141 similar to NDR-480-48 or equal  
142 (13) Serial connection with cable for local management of the device.  
143 (14) Shall operate in an environment with relative humidity of 5% to 95%  
144 (non-condensing).  
145 (15) Shall operate in an environment with ambient temperature range of –  
146 40° C to +75° C without the assistance of fan-forced cooling.  
147 (16) Shall be DIN rail mountable.  
148 (17) Lifetime manufactures warranty.  
149

150 **(D) Cellular Modem.** Procure, configure, and install a single environmentally  
151 hardened cellular modem and all required accessories with static IP addresses  
152 at the Uluoa St intersection for traffic signal control system communication only.  
153 Cellular modem shall be capable of accessing the FirstNet Band 14 as well as  
154 the 5G cellular network when available. The modem shall provide  
155 communication between the fiber optic interconnect system and HDOT’s traffic  
156 signal central system.  
157

158 The cellular modem shall include or comply with the following:  
159

- 160 (1) Two Ethernet LAN/WAN ports, minimum.  
161 (2) USB 2.0 or better port.  
162 (3) Two cellular antenna connectors with antennas and required accessories.  
163 (4) GPS antenna connector with antenna and required accessories.  
164 (5) Operating temperature range: -22°F to 158°F.  
165 (6) Storage temperature range: -40°F to 185°F.  
166 (7) Operating and storage humidity (non-condensing) ranges: 5% to 95%.  
167 (8) Ingress protection compliant with IP64.  
168 (9) Networking.  
169 1. IPsec Tunnel — up to ten concurrent sessions  
170 2. IKEv2 support (includes MOBIKE)  
171 3. Access Control Lists  
172 4. NAT  
173 5. NAT-less Routing  
174 (10) Security.  
175 1. 802.1x authentication for Ethernet  
176 2. Certificate support  
177 3. Application-level gateways  
178 4. MAC Address Filtering  
179

180 Provide FirstNet cellular modem data service, including activation and monthly  
181 data service. When the Engineer has determined the project has reached  
182 completion, the Contractor shall coordinate with the Engineer to transfer the  
183 cellular service to HDOT.  
184

185 **(E) Fiber Optic Cable.** The fiber optic cables, which will be used to transmit video  
186 and data signals, will consist of 6 or 72 single-mode fibers. See Contract Plans.  
187 Cables will be installed.

188  
189 Armored loose-tube, 6 or 72 single-mode OS2 fiber optic cable suitable for  
190 overhead or underground installation. Cable shall be 8.3/125 micron loose  
191 buffer, single-mode, step index optical fiber cable containing glass of type,  
192 SMF-28e, AFL SR-15e, or approved equal, and that meets the following  
193 specifications:

- 194 **(1)** ITU-T G.652 (Categories A, B, C and D)
- 195 **(2)** IEC Specification 60793-2-50 Type B1.3
- 196 **(3)** TIA/EIA 492-CAAB
- 197 **(4)** Telecordia GR-20

198  
199 All cables shall be free of material or manufacturing defects and dimensional  
200 non-uniformity that would;

- 201 **(1)** Interfere with the cable installation using accepted cable installation  
202 practices.
- 203 **(2)** Degrade the transmission performance and environmental resistance after  
204 installation.
- 205 **(3)** Inhibit proper connection to interfacing elements.
- 206 **(4)** Otherwise yield an inferior product.

207  
208 **(1) Mechanical and Performance Requirements.** The cable shall be a rugged  
209 all dielectric armored outdoor cable containing color coded buffer tubes with 12  
210 single mode color-coded fibers per- buffer tube, dual window (1310 nm and  
211 1550 nm) fibers with UV acrylate coating in color coded, gel-free, loose buffer  
212 tubes.

213  
214 Strand the loose buffer tubes around an all-dielectric center strength element  
215 using a reverse oscillation lay, wrapped by water blocking core separator or  
216 functional equivalent. The maximum allowable attenuation of the fiber is .35  
217 dB/km for 1310 nm and .25 dB/km for 1550 nm.

218  
219 Each buffer tube shall contain a water blocking element for water-blocking  
220 protection. The water blocking elements shall be non-nutritive to fungus,  
221 electrically non-conductive. The buffer-tube shall be gel-free.

222  
223 Apply water swellable tape longitudinally around the outside of the stranded  
224 tubes/fillers. The water swellable tape shall be non-nutritive to fungus,  
225 electrically non-conductive, and homogenous. It shall also be free from dirt and  
226 foreign matter. The cable manufacturer shall be TL 9000 registered.

227  
228 **(2) Outer Jacket.** Cables shall be all dielectric cable (with armoring) and shall  
229 be jacketed (sheathed) with black medium density polyethylene as defined by  
230 ASTM D1248, Type II, Class C, Category 4 and Grades J4, E7 and E8.

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Armored cable shall have two jackets, one molded to the outside of the armor and one that floats freely within the armor and contains the buffer tubes and other fiber optic cable construction components as required.

Apply jacketing material directly over the tensile strength members to provide mechanical protection, and to serve as the primary moisture barrier.

Design cable sheath to meet or exceed the tensile criteria defined in EIA-455-89a. Ensure the jacket or sheath is free of any holes, splits, or blisters. The cable jacket shall contain no metal elements and shall be of a consistent thickness. The cable shall contain at least one ripcord under the sheath for easy sheath removal.

**(3) Temperature.** The shipping, storage, installation, and operating temperature range of the cable shall meet or exceed -20 °F to +155 °F (-29 °C to +60°C).

**(4) Loose Buffer.** Contain single-mode fibers in a loose buffer tube. The configuration shall be dimensionally sized to minimize local stresses and micro bend losses.

The optical fiber cable shall be an approved product of the U.S. Department of Agriculture, Rural Electrification Administration in accordance with the requirements of REA-PE-90, or as otherwise indicated, and shall conform to EIA/TIA-598.

Each optical fiber shall consist of a doped silica core surrounded by a concentric silica cladding.

Buffer tubes shall be polypropylene. Include fillers in the cable core to lend symmetry to the cable cross section where needed.

**(5) Colors.** All optical fibers shall be identifiable by standard color codes as defined in EIA/TIA-598. Each fiber shall be distinguishable, from others by means of color coding and shall conform to the following EIA/TIA sequence of colors:

1. Blue	7. Red
2. Orange	8. Black
3. Green	9. Yellow
4. Brown	10. Violet
5. Slate	11. Rose
6. White	12. Aqua

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Buffer tubes containing fibers shall also be color-coded with distinct and recognizable colors according to the following sequence of colors:

1. Blue
2. Orange
3. Green
4. Brown
5. Slate
6. White

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The color formulation shall be compatible with the fiber coating and be heat stable. Color formulation shall not fade or smear or be susceptible to migration and it shall not affect the transmission characteristics of the optical fibers and shall not cause fibers to stick together.

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**(6) Cable Marking.** The fiber optic cable outer jacket shall be marked with manufacturer's name, the year of manufacture, the words "optical fiber cable", fiber count, type of fiber, and sequential linear foot markings.

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1. Repeat the markings every 3 feet.
2. The actual length of the cable shall be within -0/+1% of the length marking.
3. The marking shall be in a contrasting color to the cable jacket.
4. The marking shall be 2.5 mm in height and must be permanent weatherproof and shall not wear off during the installation in the underground conduit system.

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**(7) Quality Assurance Provision.** The fiber optic cable shall meet or exceed the requirements of this specification when measured in accordance with the methods of the individual requirements or the following methods as defined in EIA-455-A:

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1. Fiber dimensions
2. Attenuation
3. Numerical aperture
4. Fiber proof test
5. Crush resistance
6. Cable bending
7. Tensile load
8. Impact resistance
9. Attenuation vs. Temperature

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**(8) Packaging.** Top and bottom ends of the cable shall be available for testing.



308 Both ends of the cable shall be sealed to prevent the ingress of moisture.  
309 Each reel shall have a weather resistant reel tag attached identifying the reel  
310 and cable.

311  
312 The reel tag shall include the following information:

- 313 1. Cable number
- 314 2. Gross Weight
- 315 3. Shipped length in meters
- 316 4. Job order number
- 317 5. Product Number
- 318 6. Date cable tested

319  
320 Each cable shall be accompanied by a cable data sheet. Cable data shall  
321 include manufacturer number, billable length, bandwidth specs and measured  
322 attenuation of each fiber.

323

### 324 **(9) Construction Requirements.**

325

326 **Material Sample and Certificate of Compliance.** The Contractor  
327 shall submit material samples according to Subsection 106.04 –  
328 Material Sample, and any certificates of compliance according to  
329 Subsection 106.07 – Certificate of Compliance.

330

331 The Contractor shall submit a fiber optic cable pulling plan for review  
332 and approval by the Engineer prior to beginning fiber optic cable  
333 installation. The fiber optic cable pulling plan shall include:

334

- 335 (1) Location of start and end of pulls,
- 336 (2) Location of cable reel trailers during installation,
- 337 (3) Location of any “figure-eight” of fiber optic cable, and
- 338 (4) Location of staged equipment.

339

340  
341 Upon completion of the work, submit an “As Built” or corrected plan  
342 showing in detail the following:

343

- 344 (1) Construction changes,
  - 345 (2) Location and attenuation of every event along the installed fiber  
346 optic cable,
  - 347 (3) Index of refraction of installed fiber,
  - 348 (4) Fiber optic cable index of refraction, and
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- (5) Sequential fiber optic cable markings at each pullbox, cabinet, and splice closure.

The fiber optic cable Subcontractor shall install the new fiber optic cable underground in conduits as shown on the plans. The Contractor will be responsible for furnishing and pulling the new fiber in PVC ductlines using a breakaway swivel to prevent exceeding the tensile load during installation.

All fiber optic splices shall be fusion splices. Mechanical splices shall not be used. Fiber optic splice locations are permitted only at splice points where splice cabinets are shown on the plans. Fiber optic fibers shall be spliced in every splice cabinet location, and it is the responsibility of the Contractor to maintain a continuous run throughout the system. The Contractor shall leave a minimum of 20-feet of cable service loops at every cabinet and 10 feet at every pullbox.

Provide documented historical cable pulling data indicating tensile forces exerted on the cable during the installation. Any tension measurements, which exceed the manufacturer's recommendation, will be considered means for the cable rejection. The fiber optic cable Subcontractor shall be fully responsible for the quality and integrity of the installed cable and the operability of the final fiber optic cable product. All fibers shall be spliced at camera cabinets, hubs, and splice cabinets and shall have no more than 0.07 dB loss per splice based on the appropriate system operating wavelength.

The Contractor shall complete all required fiber optic splices prior to final testing and acceptance. As part of the final testing and acceptance, submit optical time domain reflectometer (OTDR) readings in both hardcopy and electronic formats (such that it can be examined using the manufacturer's OTDR software) to the Engineer for review. Testing shall be conducted on all single mode fibers at 1310 nm and 1550 nm from the beginning and end of entire run; which includes patch panels and splicing. Powermeter attenuation testing should be performed at dual wavelength, bi-directionally.

All necessary equipment and plug-in, fiber optic pigtails, fittings, splice tags, enclosures, and work to complete an operational system shall be furnished and installed by the Contractor, unless otherwise indicated, at no added cost, and will be considered included in the cost of the contract items in this Section.

**(F) Interconnect Fabric Subduct.**

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**(1) Description** Raceway Innerduct shall be installed in all new and existing raceways containing 6 and 72 strand fiber optic cables. A non-metallic flexible textile raceway known as interconnect fabric subduct, which is placed within PVC conduits. The interconnect fabric subduct allows for future communication upgrades, including transitioning from multipair copper cables to fiber optic media. To further that effort and achieve maximum conduit utilization, all new and empty existing conduits containing the interconnect/fiber optic cables shall contain an interconnect fabric subduct. The interconnect fabric subduct shall consist of flexible, textile material, sometimes referred to as "fabric duct".

**(2) Fabric** The interconnect fabric subduct shall consist of the following:

**(a)** Standard Outdoor Textile subduct: Micro (33mm), 2-inch, 3-inch and 4-inch multi-cell polyester/nylon textile subduct containing 1,250 lb polyester flat woven pull tape.

Number of cells shall be the maximum number allowed for the conduit size.

**(b)** Conduit Plugs: Compression-type conduit plugs with locking nuts for sealing and securing one or more textile subducts within a conduit.

**(c)** C. Pull Tape: The subduct pull tape shall be constructed of synthetic fiber, printed with accurate sequential footage marks and color-coded.

**(d)** D. Duct Water Seal: products suitable for closing underground and entrance conduit openings where subduct is installed, to prevent entry of gases, liquids, or rodents into the structure.

**(3) Installation** The contractor shall protect the interconnect fabric subduct from the effects of moisture, UV exposure, corrosion and physical damage during installation. The contractor shall install the interconnect fabric subduct prior to installing the new interconnect and fiber optic cables.

The contractor shall provide interconnect fabric subduct in conduits using continuous unspliced lengths of interconnect fabric subduct between pull boxes, and/or termination points as indicated on the drawings.

The contractor shall make a 2" incision, approximately 18" from the end of interconnect fabric subduct. Pull out and cut off approximately 2 feet of pull-tape. Thus, allowing the pull tape ends to retract back into the cells.

Using approximately 6 feet of pull tape, tie a non-slip knot to the incision. Then tie 3 to 6 half-hitch knots down to the end of interconnect fabric subduct. Apply black vinyl tape over all knots and the end of interconnect fabric subduct. Using a Bow Line knot tie a swivel to the end of 3 feet pull

451 tape. For multi-pack installations one swivel is sufficient but stagger each  
452 interconnect fabric subduct.

453  
454 Using a Bow Line knot, attach the pull rope located in the rigid conduit to  
455 the other end of the swivel. Install interconnect fabric subduct - ensuring  
456 that no twist is introduced to the interconnect fabric subduct.

457  
458 Provide suitable interconnect fabric subduct slack in the pull boxes, and  
459 at turns to ensure there is no kinking or binding of the product.

460  
461 At locations where interconnect fabric subduct will be continuous through  
462 a pullbox, allow sufficient slack so that the interconnect fabric subduct  
463 may be secured to the side of the pullbox maintaining the minimum  
464 bending radius.

465  
466 At pullboxes serving as the junction location, pull the exposed end of the  
467 interconnect fabric subduct to the far end of the pullbox, install  
468 termination bag, and secure to the pullbox.

469  
470 Seal all conduit and interconnect fabric subduct entering the pullboxes to  
471 prevent entrance into the pullboxes of gases, liquids or rodents.

472

473 **627.03 EXISTING TRAFFIC SIGNAL CONTROLLER FIBER INTERFACE.** At the  
474 Kalaniana'ole Highway/Kailua Road (Waimanalo Junction) intersection, the  
475 Contractor shall install a signal controller fiber interface within the existing traffic  
476 signal cabinet. The signal controller fiber interface shall include, but not limited to, a  
477 hardened ethernet switch, (see Section 627.02(C)) and a fiber splice enclosure  
478 which shall be able to fit in the spare space within the existing traffic signal cabinet.  
479 The traffic signal controller fiber interface shall allow the existing traffic signal  
480 controller to be interconnected with the two new signals.

481  
482 **627.04 CCTV TRAFFIC CAMERA ASSEMBLY.** The camera assemblies are for  
483 traffic monitoring and traffic signal operations at the H-3 Traffic Operations Center  
484 (TOC) and/or Joint Traffic Management Center (JTMC). The CCTV cameras shall  
485 be directly connected to the cellular modems via an outdoor rated CAT 6 Ethernet  
486 cable. Contractor shall supply two CAT 6 cables between the modem and the CCTV  
487 cameras; one as a spare. It shall be an integrated camera unit consisting of a  
488 receiver, pan & tilt, housing, and cables built as a single assembly having 360  
489 degree of continuous pan rotation. The camera shall have full HD 1080p 30 image  
490 resolution with integral 30x optical zoom lens. The positioning device shall include  
491 true day-night with variable speed pan and tilt technology with a minimum sensitivity  
492 of 0.0 lux @30 IRE. The camera shall provide up to 5 independent output video  
493 streams configurable for H.264 and MJPEG and analog video output, electronic  
494 image stabilization, and wide dynamic range. Camera assembly shall be furnished  
495 with components assembled, complete, and a ready-to-install system. Camera  
496 system shall meet FHWA's Buy America requirement.

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498 **(A) CCTV Camera**

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## **(1) CAMERA IMAGING**

- (a)** Image Sensor: Progressive Scan CMOS
- (b)** Image Size: Diagonal 6mm
- (c)** Image Resolution: 1920 horizontal x 1080 vertical pixels
- (d)** Picture Elements (total) 1920 (H) x 1440 (V)
- (e)** Sensitivity: Scene Illumination; F1.4 @ 50% Video
  - (1)** 0.4 Lux (0.04 fc) @ 1/30 shutter, color mode
  - (2)** 0.0025 Lux (0.00025 fc) @ 1/2 shutter, mono mode
- (f)** Day/Night Operation: Adjustable (Auto, Color and Mono Modes)
- (g)** Optical Zoom Range: 30x, minimum
- (h)** Digital Zoom: 1x to 12x in 1x increments. The camera system shall support digital zoom limit setting
- (i)** Auto Focus: Selectable Auto/Manual; Minimum Scene Illumination for Reliable Auto Focus shall be no more than 50% video output.
- (j)** Auto Iris; Selectable auto/manual; Iris shall automatically adjust to compensate for changes in scene illumination to maintain constant video level output.
- (k)** Electronic Image Stabilization: Shall support On/Off mode
- (l)** Backlight Compensation: Shall support On/Off mode
- (m)** White Balance: Shall support Auto/Manual mode
- (n)** IR Correction: Shall support On/Off mode
- (o)** Sharpness: Shall provide user control of increases or decreases in image sharpness through 4 user selectable settings of soft, normal, sharp and sharpest

## **(2) H.264/MJPEG ENCODING ENGINE**

- (a)** The video encoding shall allow the following possible video stream configurations:
  - (1)** H.264 Streams: (1) 1920x1080 @ 30fps, (1) 1280x720 @ 30 fps, 720x480 @ 15 fps
  - (2)** MJPEG Streams: 1920x1080 @ 10 fps, 1280x720 @ 20 fps
  - (3)** Analog Video Output: (1).
- (b)** Each video encoder channel shall provide the following configurable properties;
  - (1)** Codec.
  - (2)** Video frame shall be adjustable from 30 fps to 1 fps in increments of 1 fps.
  - (3)** Bite Rate control
- (c)** Video Stream Protocols; the camera system shall support the following protocols:
  - (1)** RTSP/RTP; The RTSP communication shall occur over a TCP socket. RTP video packets shall be sent over UDP.
  - (2)** RTSP Interleaved; RTSP commands and the RTP video packets shall be transmitted over a single TCP connection.

- 546 (3) HTTP tunneling; this mode shall use two separate TCP  
547 connections for sending and the other for received data from  
548 the client over port 80  
549 (4) RTP multicast; this mode shall send RTP video packets to the  
550 user assigned multicast destination. This mode shall be  
551 required to be enabled or disabled.  
552 (d) Network Protocol Layers: TCP, UDP, IPv4, IGMP, ICMP, DNS,  
553 DHCP, RTP, RTSP, NTP, HTTP, HTTPS, ARP, and ONVIF Profile  
554 S as a minimum.  
555

### 556 (3) PAN AND TILT DRIVE UNIT SPECIFICATIONS

- 557  
558 (a) Pan Movement; 360 degrees continuous rotation.  
559 (b) Pan Speed; Variable from 0.05 to 45 degrees/second .  
560 (c) Pan Repeatability; +/- 0.05 degree precision.  
561 (d) Pan Preset Speed; 180 degree movement 2.5 < Seconds.  
562 (e) Tilt Movement; Minimum of +90 to -90 degrees.  
563 (f) Tilt Speed; Variable from 0.05 to 45 degrees/second.  
564 (g) Tilt Repeatability; +/- 0.05 degree precision.  
565 (h) Tilt Preset Speed; 180 degree movement < 2.5 Seconds.  
566 (i) Proportional Zoom Control; Positioning control shall allow variable  
567 pan/tilt. speeds based on zoom position.  
568 (j) Home Position: Shall be a user defined point.  
569 (k) The Inter Process Communication System (IPCS) shall not have  
570 any exposed wiring from the positioning drive to the camera head  
571 enclosure.  
572

### 573 (4) Electrical

- 574  
575 Operating Voltage; The camera system shall provide flexible power  
576 input as required by the installation to include:  
577  
578 (a) Power over Ethernet, LTPoE++.  
579 (b) Power injector  
580

### 581 (5) Certifications/Ratings

- 582  
583 (a) FCC Class A.  
584 (b) International Electrotechnical Commission (IEC) / European  
585 Conformity (CE) cover product emission and immunity  
586 requirements (CISPR) 22 24.  
587 (c) Restriction of Certain Hazardous Substances (RoHs)  
588

### 589 (6) Enclosure

- 590  
591 (a) Aluminum

- 592 (b) Dust-tight
- 593 (c) Waterproof & Pressurized

594  
595 **(7) Controls**

596  
597 Shall be controllable or interoperable by a Pelco analog switcher and  
598 control System using Pelco P protocol IP protocol shall be controllable  
599 by either Pelco P or Onvif protocol.

600  
601 **(8) Adapter Plate**

602  
603 A Stainless Steel, ¼” minimum, adapter plate shall be provided to  
604 integrate the supplied camera mounting to the existing mounting.

605  
606 **(9) Warranty**

607  
608 Manufacturer’s warranty period shall be three (3) years minimum.

609  
610 **Mount**

- 611 • Outdoor type
- 612 • Aluminum or stainless steel components
- 613 • Mount cantilever style on pole shafts using straps, or on horizontal mast arm shaft
- 614 • Constructed of marine grade stainless steel
- 615 • Has cable feed-through
- 616 • Supports up to 100 lbs
- 617 • Painted White
- 618 • Wall to pole mount adapter, as required
- 619 • Provide ability to level and adjust camera to plumb

620  
621 **(B) Cellular Modem**

622 Procure, configure, and install environmentally hardened cellular modems  
623 and all required accessories with static IP addresses. Cellular modems shall  
624 be capable of accessing the FirstNet Band 14 as well as the 5G cellular  
625 network when available. The modem shall provide communication between  
626 the CCTV cameras and HDOT’s CCTV systems up at the H-3 TOC. A  
627 modem specifically designated for CCTV communication only will be installed  
628 for each new CCTV camera. One modem will be installed at the Uluaa St  
629 intersection and another will be installed at the Ulumanu Dr intersection.

630  
631 All cellular modems shall include or comply with the following:

- 632 **(1)** Two Ethernet LAN/WAN ports, minimum.
- 633 **(2)** USB 2.0 or better port.
- 634 **(3)** Two cellular antenna connectors with antennas and required accessories.
- 635 **(4)** GPS antenna connector with antenna and required accessories.
- 636 **(5)** Operating temperature range: -22°F to 158°F.
- 637 **(6)** Storage temperature range: -40°F to 185°F.

638 (7) Operating and storage humidity (non-condensing) ranges: 5% to 95%.  
639 (8) Ingress protection compliant with IP64.  
640 (9) If wifi capable, the modem shall be able to disable the wifi capabilities.  
641 (10) Networking.

- 642 (a) IPsec Tunnel — up to ten concurrent sessions
- 643 (b) IKEv2 support (includes MOBIKE)
- 644 (c) Access Control Lists
- 645 (d) NAT
- 646 (e) NAT-less Routing

- 647
- 648 (11) Security.
- 649 (a) 802.1x authentication for Ethernet
  - 650 (b) Certificate support
  - 651 (c) Application-level gateways
  - 652 (d) MAC Address Filtering

653

654 Provide FirstNet cellular modem data service, including activation and monthly  
655 data service to provide 24/7 video to the H-3 TOC. When the Engineer has  
656 determined the project has reached completion, the Contractor shall coordinate  
657 with the Engineer to transfer the cellular service to HDOT.

658

659 **627.05 MEASUREMENT.** Traffic Signal Control System and Existing Traffic Signal  
660 Controller Fiber Interface will be paid on a lump sum basis. Measurement for payment  
661 will not apply.

662

663 The Engineer will measure CCTV Traffic Camera Assembly per each, in accordance  
664 with the contract documents, complete in place.

665

666 **627.06 PAYMENT.** The Engineer will pay for the accepted Traffic Signal Control  
667 System, complete in place, on a lump sum basis. The price shall include furnishing and  
668 installing server licenses; CCTV cabinets, conduits and foundations; modems; switches  
669 with SFP modules; fiber optic cables and splice trays; cables; splicing; OTDR testing  
670 and furnishing results; furnishing and installing any additional items and all tools, labor,  
671 equipment, and incidentals necessary to complete the work.

672

673 The Engineer will pay for the accepted Existing Traffic Signal Controller Fiber Interface,  
674 complete in place, on a lump sum basis. The price shall include furnishing and  
675 installing the items, and all tools, labor, equipment, and incidentals necessary to  
676 complete the work.

677

678 The Engineer will pay for accepted quantities of the CCTV Traffic Camera Assembly at  
679 the contract unit price per each completed in place. The price shall include CCTV  
680 cameras; modems; cables; splicing; making the connections; testing; providing turn-on  
681 service; furnishing and installing any additional items, and all tools, labor, equipment,  
682 and incidentals necessary to complete the work.

683



684 The Engineer will pay for the FirstNet cellular modem data service activation and the  
685 three cellular modem data service monthly costs. These items will be paid for on a  
686 force account basis in accordance with Subsection 109.6 – Force Account Provisions  
687 and Compensation.

688  
689 The Engineer will consider full compensation for additional materials and labor not  
690 specifically shown or called for that are necessary to complete the work incidental to the  
691 various contract items in the proposal. The Engineer will pay for each of the following  
692 pay items when included in the proposal schedule:

693	<b>Pay Item</b>	<b>Pay Unit</b>
694		
695		
696	Traffic Signal Control System	Lump Sum
697		
698	Existing Traffic Signal Controller Fiber Interface	Lump Sum
699		
700	CCTV Traffic Camera Assembly	Each
701		
702	Cellular Modem Data Service	Force Account
703		

704  
705

**END OF SECTION 627**



42

Cladding Diameter	125 μm ± 1.0 μm
Core-to-Cladding Offset	Less than 0.6 μm
Cladding Non-Circularity	Less than 1.0%
Coating Diameter	245 μm ± 10 μm
Colored Fiber Diameter	Nominal 250 μm
Attenuation Uniformity	Attenuation Uniformity No point discontinuity greater than 0.10 dB at either 1310 nm or 1550 nm
Attenuation at the Water Peak	The attenuation at 1383 ± 3 nm shall not exceed 2.1 dB/kM
Cutoff Wavelength	The cabled fiber cutoff wavelength shall be less than 1260 nm
Mode-Field Diameter 9.30 ± 0.50 μm at 1310 nm	9.30 ± 0.50 μm at 1310 nm 10.50 ± 1.00 μm at 1550 nm
Zero Dispersion Wavelength	Less than 1301.5 nm
Zero Dispersion Slope	Less than 0.092 ps/(nm <sup>2</sup> kM)
Fiber Polarization Mode Dispersion	Less than 0.5ps/kM

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The coating shall be a dual layered, UV cured acrylate applied by the fiber manufacturer. The coating shall be mechanically strippable.

45

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**(B) Fiber Specification Parameters.**

48

Required Fiber Grade	Maximum individual fiber attenuation
Single Mode	The maximum dispersion shall be less than or equal to 3.2 ps/nm <sup>2</sup> kM from 1285 to 1330 nm and shall be less than 18 ps/nm <sup>2</sup> -kM at 1550 nm

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50

The fiber manufacturer shall proof test all optical fibers to a minimum load of 0.7 GN/m<sup>2</sup> (100 kpsi).

51

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53

**(C) Specifications for Outdoor Cable Construction.** Optical fibers shall be inside a loose buffer tube in groups of 12. Optical fibers shall be mechanically strippable. Do not use gel filled. The fiber shall be colored with ultraviolet (UV) curable links. Each fiber shall be distinguishable by means of color coding in accordance with TIA/EIA-598- A, "Optical Fiber Cable Color Coding."

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Loose buffer tubes shall also be colored with distinct and recognizable colors in accordance with TIA/EIA-598-A, "Optical Fiber Cable Color Coding" and shall be marked Single mode. Fillers may be included in the cable core to lend symmetry to the cable cross section where needed. Cable construction shall utilize dielectric strength members.

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Cable jacket shall be a PVC material that is fungus, water and UV resistant. The jacket shall be marked with the manufacturer's name, sequential meter or foot marking, month and year of manufacture,.

The maximum pulling tension shall be 2700 N (608 lbft) during installation (short term) and 890 N (200 lbft) long term installed.

The shipping, storage, and operating temperature range of the cable shall be -40C to +70C.

**(D) Quality Assurance Provision.** All cabled optical fibers > 1000 meters in length shall be 100% attenuation tested. Attenuation of each fiber shall be provided with each cable reel.

The cable manufacturer shall be ISO 9001 registered.

**(E) Packaging.** Top and bottom ends of the cable shall be available for testing.

Both ends of the cable shall be sealed to prevent the ingress of moisture. Each reel shall have a weather resistant reel tag attached identifying the reel and cable.

The reel tag shall include the following information:

Cable number	Gross Weight
Shipped length in meters	Job order number
Product Number	Date cable tested

A cable data sheet shall accompany each cable. Cable data shall include manufacturer number, billable length, bandwidth specs and measured attenuation of each fiber.

**647.03 Construction Requirements.**

**(A) Drawings.** The Contractor shall submit a fiber optic cable-pulling drawings for review and acceptance by the Engineer prior to beginning fiber optic cable installation. Do not install fiber optic cable without the Engineer's acceptance of the pulling drawings. The fiber optic cable pulling drawings shall include:

- (1) Location of start and end of pulls,
- (2) Location of cable reel trailers during installation,

- 112 (3) Location of any "figure-eight" of fiber optic cable, and  
113  
114 (4) Location of staged equipment.  
115

116 Upon completion of the work, submit an 'As Built' in accordance  
117 with Subsection 108.13(B) – Pre-Final Inspection and Section 648 – Field  
118 Posted Drawings including in detail the following:  
119

- 120 (1) Location and attenuation of every event along the installed  
121 fiber optic cable,  
122  
123 (2) Index of refraction of installed fiber,  
124  
125 (3) Fiber optic cable index of refraction, and  
126  
127 (4) Sequential fiber optic cable markings at each pullbox,  
128 cabinet, and splice closure.  
129

130 **(B) Excavation and Backfill.** Excavation and backfill shall conform to  
131 Section 206A - Excavation and Backfill for Miscellaneous Facilities.  
132

133 The Contractor and Installer shall be responsible for the repair of  
134 any damage to pavements, sidewalks and other improvements. Place the  
135 material from the excavation to prevent damage and obstruction to  
136 vehicular and pedestrian traffic and interference with surface drainage.  
137

138 **(C) Fiber Optic Cable.** The Installer shall install the new fiber optic  
139 cable overhead on existing power poles and underground in conduits as  
140 shown in the contract documents. The Contractor and Installer will be  
141 responsible for all work and equipment required to install the messenger  
142 cable (when there is not already existing messenger cable) on existing  
143 joint poles for the overhead portion of the fiber installation. For the  
144 underground portion, the Installer will be responsible for furnishing and  
145 pulling the new fiber in ductlines using a breakaway swivel to prevent  
146 exceeding the tensile load during installation.  
147

148 All fiber optic splices shall be fusion splices. Do not use mechanical  
149 splices. Fiber optic splice locations are permitted only at splice points  
150 where splice cabinets are shown on the plans. Fiber optic fibers shall be  
151 spliced in every splice cabinet location, and it is the responsibility of the  
152 Contractor and Installer to maintain a continuous run throughout the  
153 system. The Installer shall leave a minimum of 20-feet of cable service  
154 loops at every cabinet or splice location, or utilize aerial cable snowshoes  
155 for overhead storage.  
156

157 Provide documented historical cable pulling data indicating tensile  
158 forces exerted on the cable during the installation. Any tension  
159 measurements, which exceed the manufacturer's recommendation, will be  
160 considered means for the cable rejection. The Contractor and Installer  
161 shall be fully responsible for the quality and integrity of the installed cable  
162 and the operability of the final fiber optic cable product.

163  
164 All fibers shall be spliced at camera cabinets, hubs, and splice cabinets  
165 and shall have no more than 0.07 dB loss per splice based on the  
166 appropriate system operating wavelength.

167  
168 The Installer shall complete all required fiber optic splices prior to  
169 final testing and acceptance. As part of the final testing and acceptance,  
170 submit optical time domain reflectometer (OTDR) readings in both  
171 hardcopy and electronic formats (such that it can be examined using the  
172 manufacturer's OTDR software) to the Engineer for review. Testing shall  
173 be conducted on all single mode fibers at 1310nm and 1550nm. Power  
174 meter attenuation testing should be performed at dual wavelength, bi-  
175 directionally.

176  
177 All necessary equipment and plug-in, fiber optic pigtails, fittings,  
178 enclosures, and work to complete an operational system shall be  
179 furnished and installed by the Installer, unless otherwise indicated, at no  
180 increase in contract price or contract time.

181  
182 **(D) Services Provided By The County.** The City and County of  
183 Honolulu, Department of Transportation Services (DTS) will be  
184 responsible for all splices and connections in DTS pullboxes and DTS  
185 cabinet locations where indicated in the contract documents.

186  
187 The Contractor and Installer shall be responsible for the following:

188  
189 **(1)** Arrange for phases of work with DTS or as specified by the  
190 Engineer.

191  
192 **(2)** Give at least seven calendar days of advance notice to DTS  
193 when phases of the work require its services.

194  
195 **(E) Restoring Pavements and Other Improvements.** Restore the  
196 existing pavements and other improvements such as driveways,  
197 sidewalks, curbs and gutters disturbed by excavation to their original  
198 condition in accordance with the contract documents. Materials used for  
199 restoration work shall be equal to or better in quality than the materials the  
200 Contractor will replace, and matching in thickness, texture, and color  
201 whenever applicable. The grades of the restored surfaces shall conform to  
202 the existing grades.

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**(F) Warranty.** Materials and equipment installed for permanent construction shall be new. The contract contemplates the use of first-class material and equipment throughout the performance of the contract.

Secure from the manufacturer(s), a warranty or warranties guaranteeing equipment from defects in materials, design and workmanship for not less than 12 months from the date of acceptance.

When requiring adjustments or repairs during the warranty period, adjust or repair the existing unit within 24 hours from the time of notification.

When requiring repairs during the warranty period that cannot be repaired within the initial 24 hours, replace the existing unit with an accepted temporary operational replacement unit within 24 hours from the time of notification. The accepted temporary operational replacement shall remain in operation satisfactorily until the Installer can correct the problem in a manner acceptable to the Engineer or install a new unit. However, installation of the new, identical non-defective unit shall be completed within 30 days from the time of notification.

**647.04 Method of Measurement.** Fiber optic cables will be paid on a lump sum basis. Measurement for payment will not apply.

**647.05 Basis of Payment.** The Engineer will pay for the accepted fiber optic cables on a contract lump sum basis. Payment will be full compensation for the work prescribed in this section and the contract documents.

The Engineer will pay for the following pay item when included in the proposal schedule:

<b>Pay Item</b>	<b>Pay Unit</b>
Fiber Optic Cable	Lump Sum

**END OF SECTION 647**

PROPOSAL SCHEDULE

ITEM NO.	ITEM	APPROX. QUANTITY	UNIT	UNIT PRICE	AMOUNT
201.0400	Clearing and Grubbing	L.S.	L.S.	L.S.	\$ _____
202.0420	Removal of Existing Traffic Signs	L.S.	L.S.	L.S.	\$ _____
202.0050	Removal of Existing Curb and Gutter	L.S.	L.S.	L.S.	\$ _____
202.0030	Removal of Existing Sidewalk	L.S.	L.S.	L.S.	\$ _____
202.0430	Removal of Existing Grassed Median	L.S.	L.S.	L.S.	\$ _____
202.0440	Removal of Existing Pavement	L.S.	L.S.	L.S.	\$ _____
209.0100	Installation, Maintenance, Monitoring, and Removal of BMP	L.S.	L.S.	L.S.	\$ _____
209.0200	Additional Water Pollution, Dust, and Erosion Control	F.A.	F.A.	F.A.	\$ <u>50,000.00</u>
212.0100	Archaeological Monitoring	F.A.	F.A.	F.A.	\$ <u>100,000.00</u>
301.0400	Hot Mix Asphalt Base Course	L.S.	L.S.	L.S.	\$ _____
401.0400	Asphalt Concrete Pavement Mix No. IV	135	S.Y.	\$ _____	\$ _____
415.0400	Cold Planing of Existing Pavement	370	S.Y.	\$ _____	\$ _____
607.0400	Green Vinyl Coated Chain Link Fence	60	L.F.	\$ _____	\$ _____
617.0400	Imported Planting Soil	L.S.	L.S.	L.S.	\$ _____
619.0400	Arborist Services	F.A.	F.A.	F.A.	\$ <u>50,000.00</u>



**PROPOSAL SCHEDULE**

<b>ITEM NO.</b>	<b>ITEM</b>	<b>APPROX. QUANTITY</b>	<b>UNIT</b>	<b>UNIT PRICE</b>	<b>AMOUNT</b>
621.0001	Street Light Sawcut, Trench, Excavation, and Backfill	400	L.F.	\$ _____	\$ _____
621.0002	HECO Sawcut, Trench, Excavation and Backfill	150	L.F.	\$ _____	\$ _____
621.0003	HECO Metering Equipment	2	Each	\$ _____	\$ _____
621.0004	1-3" HECO Concrete Encased Conduit	100	L.F.	\$ _____	\$ _____
621.0005	1-2" Secondary Concrete Encased Conduit	50	L.F.	\$ _____	\$ _____
621.0006	1-1.5" Street Light Concrete Encased Conduit	400	L.F.	\$ _____	\$ _____
621.0007	2' x 4' HECO Handhole	2	Each	\$ _____	\$ _____
621.0008	State Street Light Standard, Base, and Single Arm and 120W Luminaire	8	Each	\$ _____	\$ _____
621.0009	State Street Light Standard, Base, and Dual Arm and 120W Luminaire	1	Each	\$ _____	\$ _____
621.0010	Street Light Conductors	400	L.F.	\$ _____	\$ _____
621.0011	Secondary Cables 2#8, #8 Gnd XHHW CU Cable	50	L.F.	\$ _____	\$ _____
621.0012	Service Cables 3#2, #8 Gnd XHHW CU Cable	10	L.F.	\$ _____	\$ _____
621.0013	Hawaiian Electric Co. Charges	F.A.	F.A.	F.A.	\$ <u>20,000.00</u>

**PROPOSAL SCHEDULE**

<b>ITEM NO.</b>	<b>ITEM</b>	<b>APPROX. QUANTITY</b>	<b>UNIT</b>	<b>UNIT PRICE</b>	<b>AMOUNT</b>
623.1000	Furnish and Install Controller Assembly with Firmware (Model 2070 Traffic Signal Controller Unit, Type 332A Cabinet and Auxiliary Equipment)	2	Each	\$ _____	\$ _____
623.2001	Type I Traffic Signal Standard, H=8 Ft	2	Each	\$ _____	\$ _____
623.2002	Type I Traffic Signal Standard, H=10 Ft	12	Each	\$ _____	\$ _____
623.2003	Type II Traffic Signal Standard With 30-Foot Mast Arm	4	Each	\$ _____	\$ _____
623.2011	Foundation For Type I Signal Standard	14	Each	\$ _____	\$ _____
623.2012	Foundation For Type II Signal Standard	4	Each	\$ _____	\$ _____
623.2013	Foundation For Controller Cabinet	2	Each	\$ _____	\$ _____
623.3001	Traffic Signal Assembly, (1-Way, 12-Inch, 1-3 Section Vertical With Type Tp-1w Mounting)	9	Each	\$ _____	\$ _____
623.3002	Traffic Signal Assembly, (1-Way, 12-Inch, 1-3 Section Vertical With Type B-1w Mounting)	3	Each	\$ _____	\$ _____
623.3003	Traffic Signal Assembly, (1-Way, 12-Inch, 1-3 Section Vertical With Type Ma-1w(1) Mounting)	8	Each	\$ _____	\$ _____
623.3004	Traffic Signal Assembly, (1-Way, 12-Inch, 1-3 Section Vertical, Programmable Visibility Head With Type With Type Tp-1w Mounting)	2	Each	\$ _____	\$ _____

**PROPOSAL SCHEDULE**

<b>ITEM NO.</b>	<b>ITEM</b>	<b>APPROX. QUANTITY</b>	<b>UNIT</b>	<b>UNIT PRICE</b>	<b>AMOUNT</b>
623.3005	Traffic Signal Assembly, (1-Way, 12-Inch, 1-3 Section Vertical, Programmable Visibility Head With Type Ma-1w(1) Mounting)	1	Each	\$ _____	\$ _____
623.3006	Traffic Signal Assembly, (2-Way, 12-Inch, 1-3 Section Vertical with Type TP-2w Mounting)	1	Each	\$ _____	\$ _____
623.3011	Evp Optical Receiver With Mast Arm Mounting	4	Each	\$ _____	\$ _____
623.3012	Evp Optical Receiver With Top Of Pole Mounting	3	Each	\$ _____	\$ _____
623.3021	Pedestrian Signal Assembly, (1-Way, 12-Inch, One Vertical With Type B-1w Mounting)	2	Each	\$ _____	\$ _____
623.3022	Pedestrian Signal Assembly, (1-Way, 12-Inch, One Vertical With Type C-1w Mounting)	7	Each	\$ _____	\$ _____
623.3023	Pedestrian Signal Assembly, (2-Way, 12-Inch, One Vertical With Type C-2w Mounting)	1	Each	\$ _____	\$ _____
623.3024	Pedestrian Signal Assembly, (1-Way, 12-Inch, One Vertical With Type Tp-1w Mounting)	1	Each	\$ _____	\$ _____
623.3025	Pedestrian Signal Assembly, (2-Way, 12-Inch, One Vertical With Type Tp-2w Mounting)	1	Each	\$ _____	\$ _____
623.4001	Pedestrian Push Button With Instruction Sign	13	Each	\$ _____	\$ _____
623.5001	Traffic Signal Ductline, One 2-Inch Conduit, Sch 40 Pvc, Concrete Encased	50	L.F.	\$ _____	\$ _____

**PROPOSAL SCHEDULE**

<b>ITEM NO.</b>	<b>ITEM</b>	<b>APPROX. QUANTITY</b>	<b>UNIT</b>	<b>UNIT PRICE</b>	<b>AMOUNT</b>
623.5002	Traffic Signal Ductline, Two 2-Inch Conduit, Sch 40 Pvc, Concrete Encased	2,600	L.F.	\$ _____	\$ _____
623.5003	Traffic Signal Ductline, Four 2-Inch Conduit, Sch 40 Pvc, Concrete Encased	400	L.F.	\$ _____	\$ _____
623.5004	Traffic Signal Ductline, Five 2-Inch Conduit, Sch 40 Pvc, Concrete Encased	250	L.F.	\$ _____	\$ _____
623.5005	Traffic Signal Ductline, Six 2-Inch Conduit, Sch 40 Pvc, Concrete Encased	200	L.F.	\$ _____	\$ _____
623.5006	Traffic Signal Ductline, Seven 2-Inch Conduit, Sch 40 Pvc, Concrete Encased	125	L.F.	\$ _____	\$ _____
623.5007	Traffic Signal Ductline, Eight 2-Inch Conduit, Sch 40 Pvc, Concrete Encased	125	L.F.	\$ _____	\$ _____
623.5008	Traffic Signal Ductline, Four 2-Inch Conduit And Two 3-Inch Conduit, Sch 40 Pvc, Concrete Encased	10	L.F.	\$ _____	\$ _____
623.5009	Traffic Signal Ductline, Four 2-Inch Conduit And Three 3-Inch Conduit, Sch 40 Pvc, Concrete Encased	10	L.F.	\$ _____	\$ _____
623.6001	Type A Pullbox	3	Each	\$ _____	\$ _____
623.6002	Type B Pullbox	31	Each	\$ _____	\$ _____
623.6003	Type C Pullbox	2	Each	\$ _____	\$ _____

**PROPOSAL SCHEDULE**

<b>ITEM NO.</b>	<b>ITEM</b>	<b>APPROX. QUANTITY</b>	<b>UNIT</b>	<b>UNIT PRICE</b>	<b>AMOUNT</b>
623.6004	Replace Type B Pullbox	2	Each	\$ _____	\$ _____
623.7001	No. 14, 2-Conductor Loop Detector Lead-In Cable	8,600	L.F.	\$ _____	\$ _____
623.7002	No. 14, 26-Conductor Traffic Control Cable	2,000	L.F.	\$ _____	\$ _____
623.7003	No. 8, 3-Conductor Power Cable	100	L.F.	\$ _____	\$ _____
623.7004	EVP Cable	1,300	L.F.	\$ _____	\$ _____
623.8001	Loop Detector Sensing Unit (6 Ft X 6 Ft) Two Loops	16	Each	\$ _____	\$ _____
623.8002	Loop Detector Sensing Unit (6 Ft X 6 Ft) Four Loops	6	Each	\$ _____	\$ _____
623.8003	Loop Detector Sensing Unit (6 Ft X 6 Ft) Six Loops	3	Each	\$ _____	\$ _____
627.1000	Traffic Signal Control System	L.S.	L.S.	L.S.	\$ _____
627.1001	Existing Traffic Signal Control Fiber Interface	L.S.	L.S.	L.S.	\$ _____
627.1002	CCTV Traffic Camera Assembly	2	Each	\$ _____	\$ _____
627.1003	Cellular Modem Data Service	F.A.	F.A.	F.A.	\$ <u>4,000.00</u>
629.0401	4-inch Pavement Striping (Tape, Type I or Thermoplastic)	300	L.F.	\$ _____	\$ _____
629.0402	4-inch Pavement Striping (Tape, Type III or Thermoplastic)	1,350	L.F.	\$ _____	\$ _____

**PROPOSAL SCHEDULE**

<b>ITEM NO.</b>	<b>ITEM</b>	<b>APPROX. QUANTITY</b>	<b>UNIT</b>	<b>UNIT PRICE</b>	<b>AMOUNT</b>
629.0403	6-inch Pavement Striping (Tape, Type II or Thermoplastic)	3,100	L.F.	\$ _____	\$ _____
629.0404	6-inch Pavement Striping (Tape, Type III or Thermoplastic)	440	L.F.	\$ _____	\$ _____
629.0405	8-inch Pavement Striping (Tape, Type I or Thermoplastic)	340	L.F.	\$ _____	\$ _____
629.0406	12-inch Pavement Striping (Tape, Type III or Thermoplastic)	245	L.F.	\$ _____	\$ _____
629.0407	Crosswalk Marking (Tape, Type III or Thermoplastic)	24	Lane	\$ _____	\$ _____
629.0408	Pavement Arrow (Tape, Type III or Thermoplastic)	20	Each	\$ _____	\$ _____
629.0409	Pavement Symbol (Paint, Tape, Type I, or Thermoplastic)	4	Each	\$ _____	\$ _____
629.0410	Type "C" Pavement Marker	86	Each	\$ _____	\$ _____
629.0411	Type "D" Pavement Marker	7	Each	\$ _____	\$ _____
629.0412	Type "H" Pavement Marker	42	Each	\$ _____	\$ _____
629.0413	Temporary Construction Zone Markings	L.S.	L.S.	L.S.	\$ _____
629.0414	Curb, 4-inch Markings (Paint) (250 L.F.)	L.S.	L.S.	L.S.	\$ _____
630.0400	Street Name Sign on Traffic Signal Mast Arm	4	Each	\$ _____	\$ _____
631.0300	Regulatory Sign (10 Square Feet or Less) with post	6	Each	\$ _____	\$ _____

**PROPOSAL SCHEDULE**

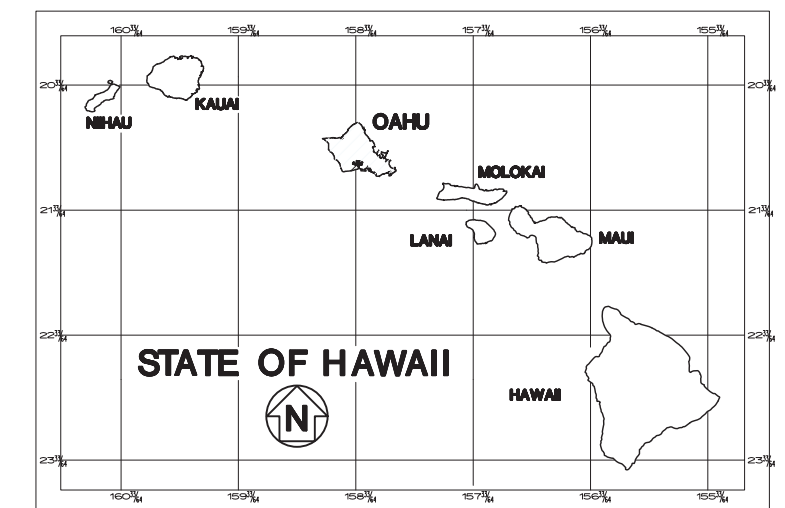
<b>ITEM NO.</b>	<b>ITEM</b>	<b>APPROX. QUANTITY</b>	<b>UNIT</b>	<b>UNIT PRICE</b>	<b>AMOUNT</b>
631.0400	Warning Sign (10 Square Feet or Less) with post	1	Each	\$ _____	\$ _____
632.0400	Type II Object Marker	7	Each	\$ _____	\$ _____
634.0400	Portland Cement Concrete Sidewalk	615	S.Y.	\$ _____	\$ _____
638.0401	Curb, Type 2D	30	L.F.	\$ _____	\$ _____
638.0402	Curb and Gutter, Type 2DG	600	L.F.	\$ _____	\$ _____
639.0401	Curb, Type 6	1,500	L.F.	\$ _____	\$ _____
639.0402	Curb, 4-inch	250	L.F.	\$ _____	\$ _____
641.0400	Hydro-mulch Seeding	L.S.	L.S.	L.S.	\$ _____
643.0100	Maintenance of Existing Landscape Areas	F.A.	F.A.	F.A.	\$ <u>25,000.00</u>
645.1000	Traffic Control	L.S.	L.S.	L.S.	\$ _____
645.2000	Additional Police Officers And/or Additional Control Device	F.A.	F.A.	F.A.	\$ <u>50,000.00</u>
648.0100	Field-Posted Drawings	L.S.	L.S.	L.S.	\$ _____
650.0401	Curb Ramp, Type "A"	7	Each	\$ _____	\$ _____
650.0402	Curb Ramp, Type "C"	2	Each	\$ _____	\$ _____

**PROPOSAL SCHEDULE**

<b>ITEM NO.</b>	<b>ITEM</b>	<b>APPROX. QUANTITY</b>	<b>UNIT</b>	<b>UNIT PRICE</b>	<b>AMOUNT</b>
650.0403	Curb Ramp, Type Combination	2	Each	\$ _____	\$ _____
650.0404	Detectable Warning Mat	13	Each	\$ _____	\$ _____
699.1000	Mobilization (Not to Exceed 6 Percent of the Sum of All Items Excluding the Bid Price of this Item)	L.S.	L.S.	L.S.	\$ _____
Sum of All Items .....					\$ _____
<p><b>NOTE:</b> Bidders must complete all unit prices and amounts. Failure to do so may be grounds for rejection of bid.</p>					



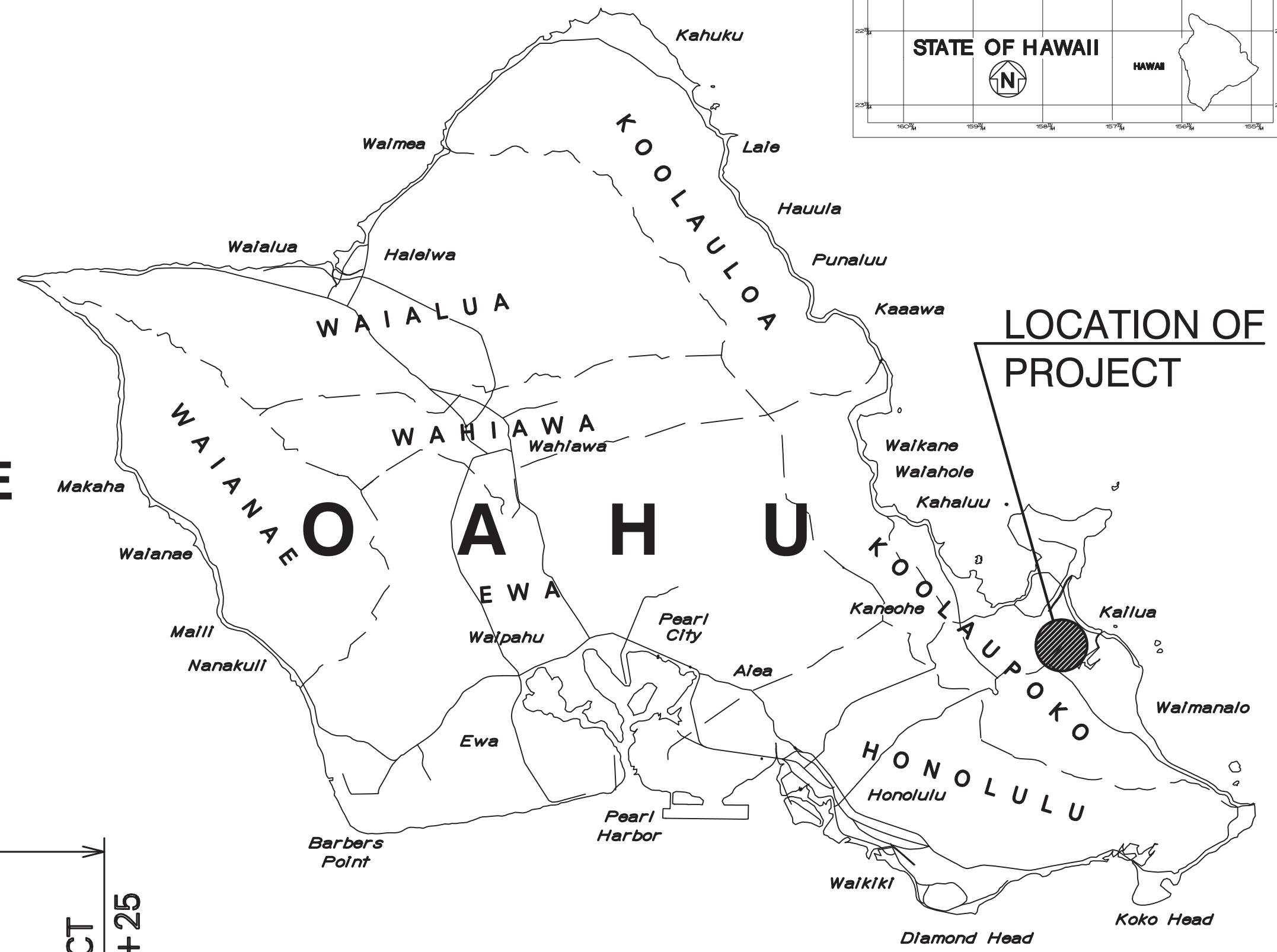
FED. ROAD DIST. NO.	STATE	PROJ. NO.	FISCAL YEAR	SHEET NO.	TOTAL SHEETS
HAWAII	HAW.	61D-01-23	2024	ADD. 1	87



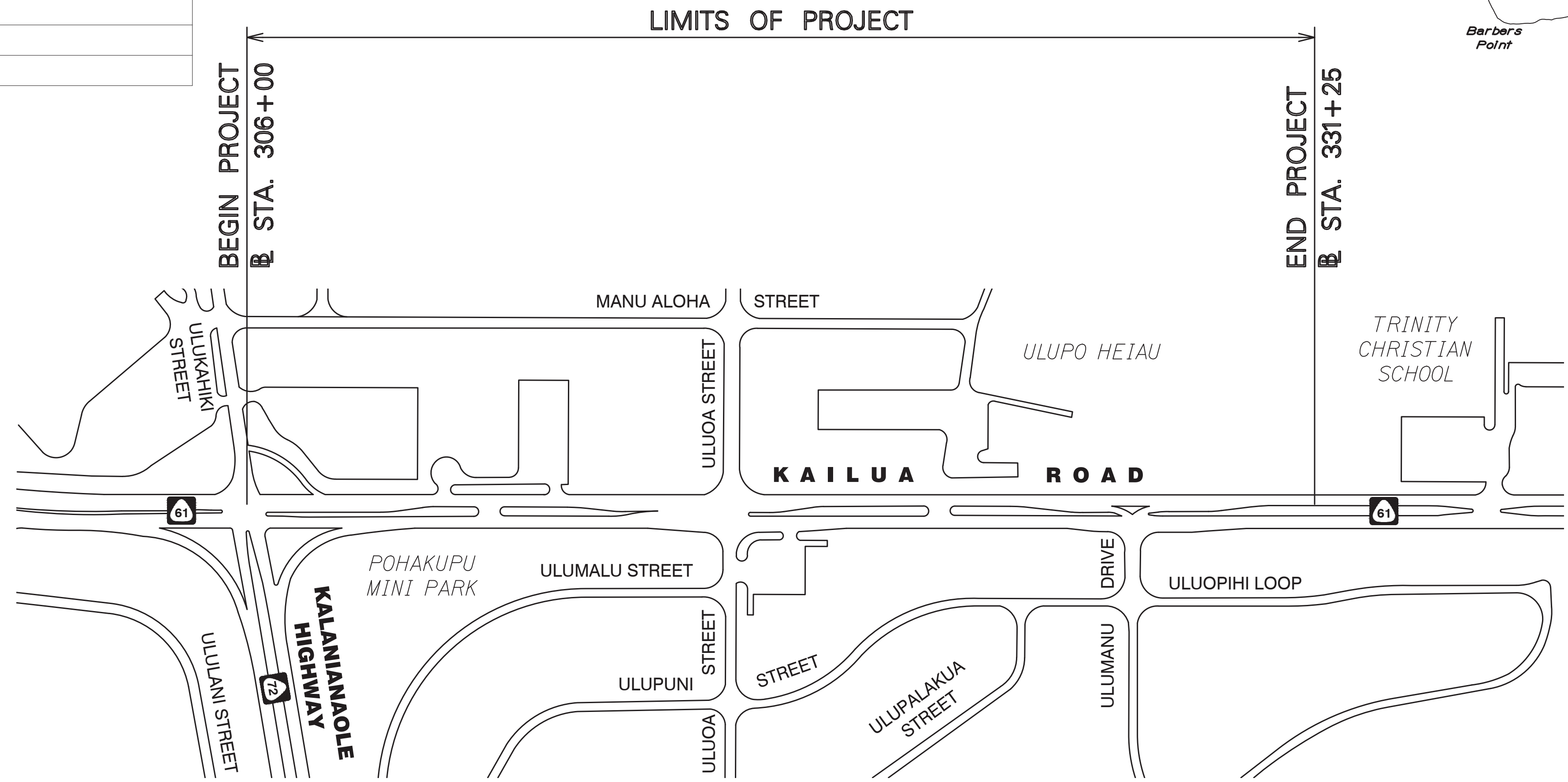
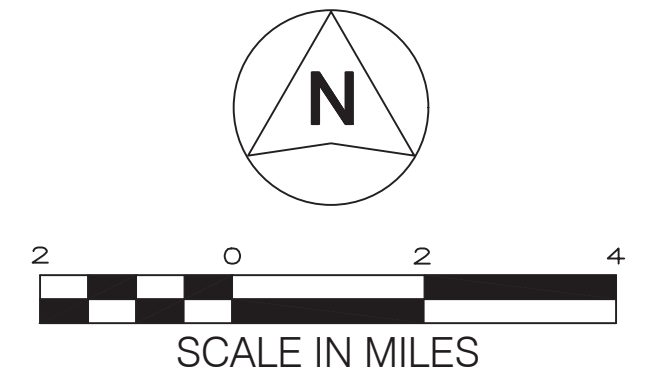
INDEX TO DRAWINGS	
SHEET NO.	DESCRIPTION
1	TITLE SHEET
2	STANDARD PLANS SUMMARY
3-7	GENERAL NOTES & LEGEND
8-10	WATER POLLUTION & EROSION CONTROL NOTES
11-12	WATER POLLUTION & EROSION CONTROL PLAN
13	SURVEY CONTROL PLANS
14-17	DEMOLITION PLANS
18-22	ROADWAY IMPROVEMENT PLANS
23-25	TYPICAL CURB RAMP NOTES & DETAILS
26-33	CURB RAMP DETAILS
34	SIGNAGE & STRIPING NOTES
35-38	SIGNAGE & STRIPING PLANS
39	PAVING NOTES & DETAILS
40	PAVING PLAN
41-42	MISCELLANEOUS DETAILS
43	MAINTENANCE OF TRAFFIC PLAN
44	TRAFFIC CONTROL NOTES
45-60	TRAFFIC CONTROL PLANS
61-64	DETOUR PLANS
65-66	TRAFFIC SIGNAL FOUNDATION DETAILS
67-68	BORING LOG
69	TRAFFIC SIGNAL DESIGN NOTES
70-75	TRAFFIC SIGNAL DESIGN PLANS
76-77	TRAFFIC SIGNAL DETAILS
78-80	ELECTRICAL NOTES
81-82	ELECTRICAL PLANS
83-87	ELECTRICAL DETAILS

STATE OF HAWAII  
DEPARTMENT OF TRANSPORTATION  
HIGHWAYS DIVISION  
OAHU, HAWAII  
PLANS FOR  
**KAILUA ROAD**  
**INTERSECTION IMPROVEMENTS**  
VICINITY OF ULUOA STREET AND ULUMANU DRIVE  
**PROJECT NO. 61D-01-23**

DISTRICT OF KOOLAUPOKO  
ISLAND OF OAHU



LOCATION OF PROJECT

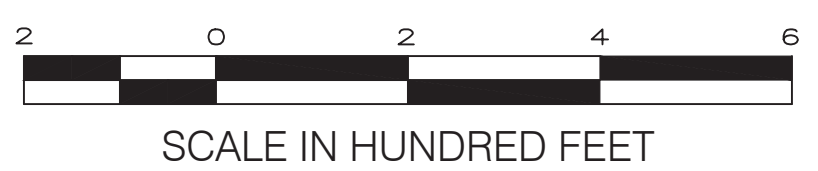


BEGIN PROJECT  
STA. 306+00

END PROJECT  
STA. 331+25

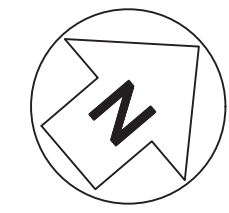
LIMITS OF PROJECT

MILE POST 9.49 TO MILE POST 9.96



**LAYOUT PLAN**

LENGTH OF PROJECT . . . . .0.5 MILES



1/11/24 DATE  
Removed CC#H Approval Block REVISION

DEPARTMENT OF TRANSPORTATION  
STATE OF HAWAII  
APPROVED: *[Signature]* Nov 3, 2023  
DIR. OF TRANSPORTATION DATE

COMMUNITY PLANNING & ENGINEERING, INC.  
DESIGNED BY  
MANAGED BY  
HWY-TO (808) 692-7691  
PHONE  
DATE DECEMBER 2023

ORIGINAL PLAN	SURVEY PLOTTED BY	DATE
NOTE BOOK	DRAWN BY	
	TRACED BY	
	DESIGNED BY	
	QUANTITIES BY	
	CHECKED BY	



FED. ROAD DIST. NO.	STATE	PROJ. NO.	FISCAL YEAR	SHEET NO.	TOTAL SHEETS
HAWAII	HAW.	61D-01-23	2024	ADD. 4	87

## CONSTRUCTION NOTES FOR WORK WITHIN CITY RIGHT-OF-WAY

- All applicable construction work shall be done in accordance with the Standard Specifications for Public Works Construction, September 1986 and Standard Details for Public Works Construction, September 1984, as amended, of the Department of Public Works, City and County of Honolulu and the Counties of Kauai, Maui and Hawaii.
- The underground pipes, cables or ductlines known to exist by the Engineer from his search of records are indicated on the plans. The Contractor shall verify the locations and depths of the facilities and exercise proper care in excavating in the area. Wherever connections of new utilities to existing utilities are shown on the plans, the Contractor shall expose the existing lines at the proposed connections to verify their locations and depths prior to excavation for the new lines.
- No Contractor shall perform any construction operation so as to cause falling rocks, soil or debris in any form to fall, slide or flow into existing City drainage systems, adjoining properties, streets, or natural watercourses. Should such violations occur, the Contractor may be cited and the Contractor shall immediately make any remedial actions necessary.
- The General Contractor/Developer/Owner of the project shall be responsible for conformance with applicable provisions of the Hawaii Administrative Rules, Title 11, Chapter 54, "Water Quality Standards," and Title 11, Chapter 55, "Water Pollution Control", as well as Chapter 18A of the Revised Ordinances of Honolulu, as amended. Best Management Practices shall be employed at all times during construction.

The General Contractor/Developer/Owner of the project shall obtain National Pollutant Discharge Elimination System (NPDES) Permit coverage(s) for the following:

- Storm water discharges associated with construction activities that disturb one (1) acre or more, and
- Discharges of hydrotesting effluent, dewatering effluent, and well drilling effluent to State waters.

In accordance with State law, all discharges related to project construction or operations are required to comply with State Water Quality Standards (Hawaii Administrative Rules, Chapter 11-54). Best Management Practices shall be used to minimize or prevent the discharge of sediment, debris, and other pollutants to State waters. Permit coverage is available from the Department of Health, Clean Water Branch at <http://health.hawaii.gov/cwb>. The Owner/Developer/Contractor is responsible for obtaining other Federal, State, or local authorizations as required by law.

- For non-City projects, the Contractor shall notify the Civil Engineering Branch, D.P.P. at 768-8084 to arrange for inspectional services and submit two (2) sets of approved Construction Plans seven (7) days prior to commencement of construction work. For City projects, the Contractor shall coordinate inspectional services with the responsible City agency.

### 6. Confined Space

For entry by City personnel, including inspectors, into a permit required confined space as defined in 29 CFR Part 1910.146(b), the Contractor shall be responsible for providing:

- All safety equipment required by the confined space regulations applicable to all parties other than the construction industry, to include, but not limited to, the following:
  - Full body harnesses for up to two personnel.
  - Lifeline and associated clips.
  - Ingress/egress and fall protection equipment.
  - Two-way radios (walkie-talkies) if out of line-of-sight.
  - Emergency (escape) respirator (10 minute duration).
  - Cellular telephone to call for emergency assistance.
  - Continuous gas detector (calibrated) to measure oxygen, hydrogen sulfide, carbon monoxide and flammables (capable of monitoring at a distance of least 20-feet away.)
  - Personal multi-gas detector to be carried by inspector.
- Continuous forced air ventilation adequate to provide safe entry conditions.
- One attendant/rescue personnel topside (two, if conditions warrant it).

- Pursuant to Chapter 6E, HRS, in the event any artifacts or human remains are uncovered during construction operations, the Contractor shall immediately suspend work and notify the Honolulu Police Department, and the State Department of Land and Natural Resources-Historic Preservation Division (692-8015). In addition, for non-City projects, the Contractor shall inform the Civil Engineering Branch, Department of Planning and Permitting (768-8084); and for City projects, notify the responsible City agency.

- For projects abutting State Highways' rights-of way, the Owner or his authorized representative shall notify the State Department of Transportation, Highways Division, Oahu District, Drainage Discharge Unit at 831-6793 for an assessment of State Highways permit requirements.

- For Bench Mark, see Sheet 13.

ORIGINAL PLAN	DATE
DESIGNED BY	
TRACED BY	
DESIGNED BY	
QUANTITIES BY	
CHECKED BY	
NO.	


APPROVALS:

CHIEF, CIVIL ENGINEERING BRANCH, D.P.P.

DATE

DIRECTOR, DEPARTMENT OF PLANNING AND PERMITTING  
CITY & COUNTY OF HONOLULU

DATE

	STATE OF HAWAII DEPARTMENT OF TRANSPORTATION HIGHWAYS DIVISION
	<b>GENERAL NOTES &amp; LEGEND - 2</b>
KAILUA ROAD INTERSECTION IMPROVEMENTS Vicinity of Uluaa Street and Ulumanu Drive Project No. 61D-01-23	
THIS WORK WAS PREPARED BY ME OR UNDER MY SUPERVISION. CONSTRUCTION OF THIS PROJECT WILL BE UNDER MY OBSERVATION. LICENSE EXPIRATION DATE: 04/30/24	Scale: N/A Date: DEC, 2023

1/11/24

△ Revised Note 4; Removed Note 6; Added CC&H Approval Block

DATE

REVISION

SHEET No. 2 OF 5 SHEETS

ADD. 4

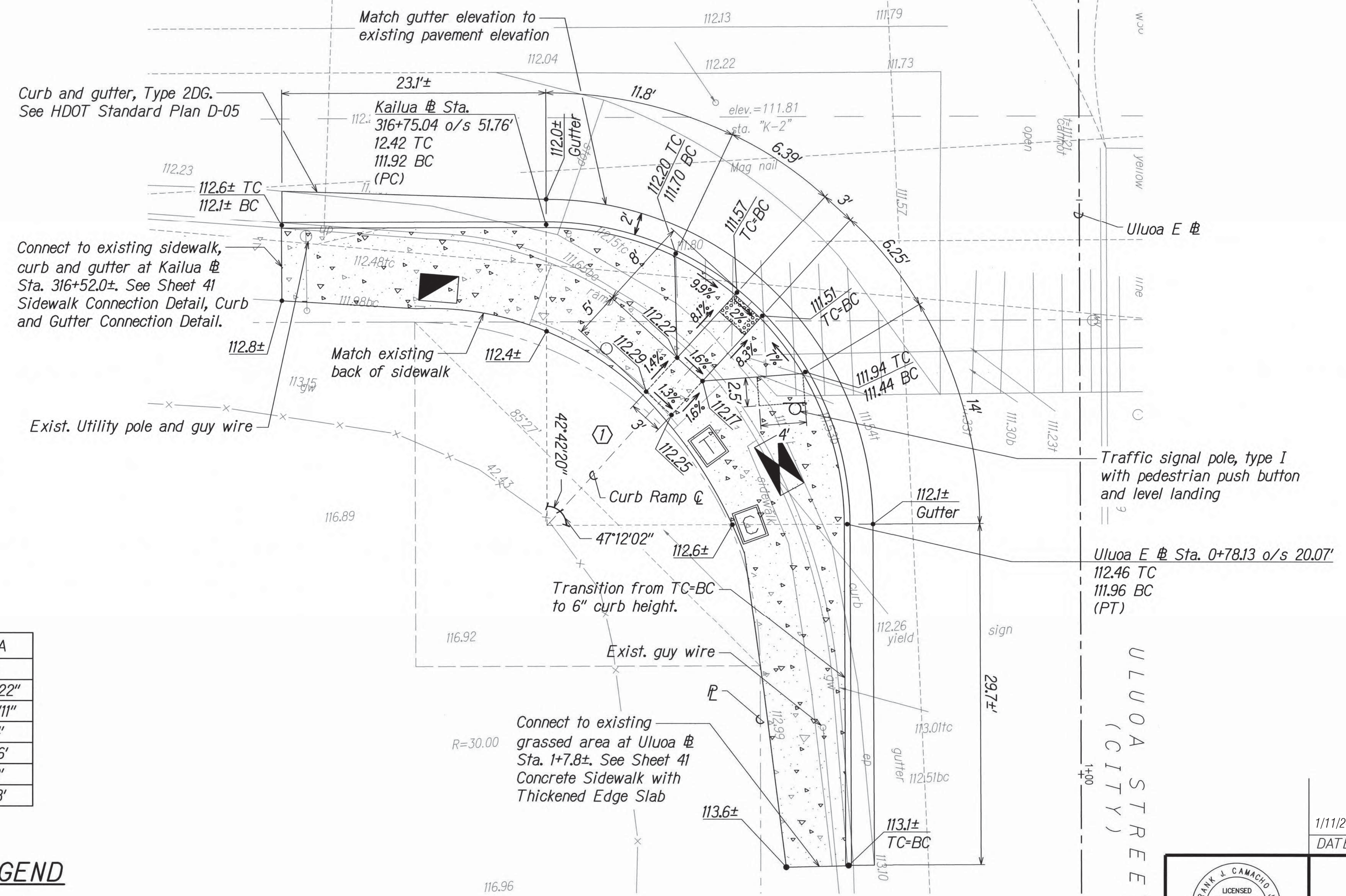
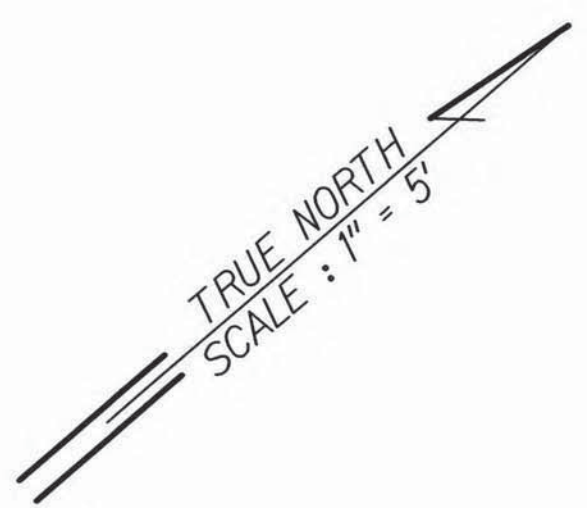


FED. ROAD DIST. NO.	STATE	PROJ. NO.	FISCAL YEAR	SHEET NO.	TOTAL SHEETS
HAWAII	HAW.	61D-01-23	2024	ADD.27	87

To Honolulu  
←

KAILUA ROAD (STATE)

To Kailua  
→

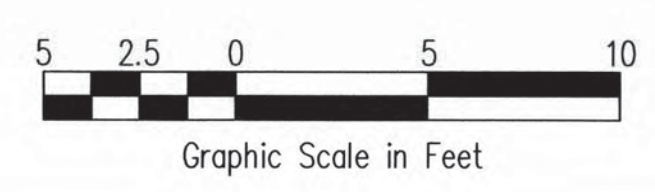


CURVE DATA	
CURVE	①
Δ	89°54'22"
Δ/2	44°57'11"
R	26.4'
T	26.36'
Ch	37.3'
L	41.43'

**LEGEND**

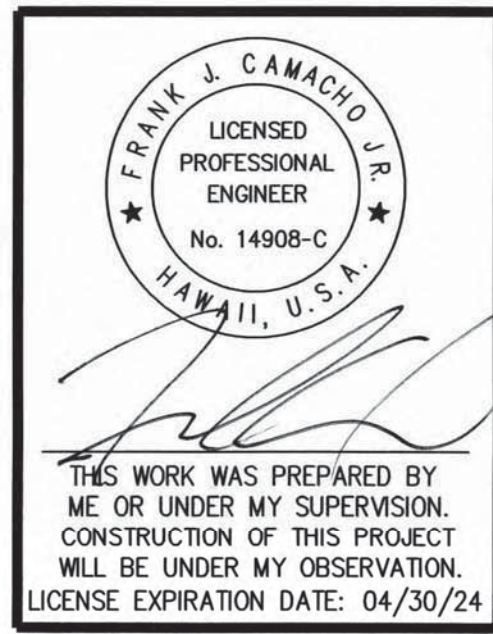
- Concrete area
- Grassed area
- Pedestrian detectable warning

**KAILUA ROAD AND ULUOA STREET INTERSECTION  
SOUTHEAST SIDEWALK PLAN**



APPROVED: \_\_\_\_\_  
DATE \_\_\_\_\_  
CHEF, CIVIL ENGINEERING BRANCH, D.P.P. (FOR CONSTRUCTION IN CITY RIGHT-OF-WAY ONLY)

1/11/24	Revised Curb Ramps
DATE	REVISION



STATE OF HAWAII  
DEPARTMENT OF TRANSPORTATION  
HIGHWAYS DIVISION

**CURB RAMP DETAILS - 2**

KAILUA ROAD INTERSECTION IMPROVEMENTS  
Vicinity of Uluoa Street and Ulumanu Drive  
Project No. 61D-01-23

Scale: 1" = 5' Date: DEC, 2023

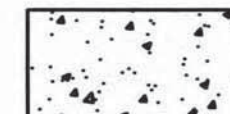
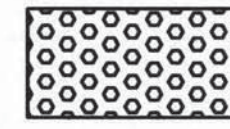
P:\Land Projects\DOT IDIQ - 2022\Traffic Operations Task\PAO 2 Kailua Traffic\Drawings\27 Curb Ramp Details - 2.dwg, 1/11/2024 4:20:15 PM

SURVEY PLOTTED BY	DATE
DESIGNED BY	
QUANTITIES BY	
CHECKED BY	
ORIGINAL PLAN	
NOTE BOOK	
No.	

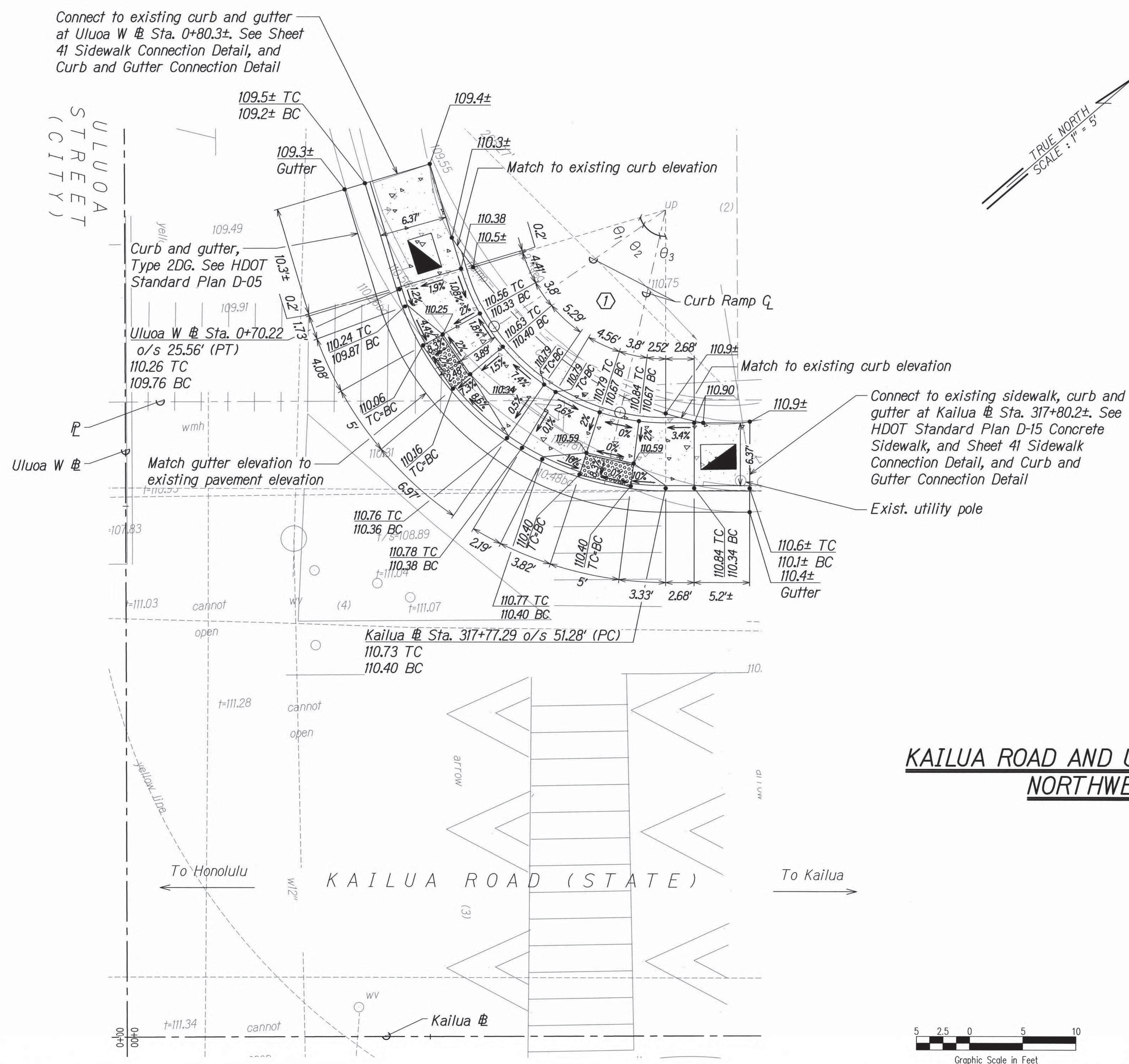
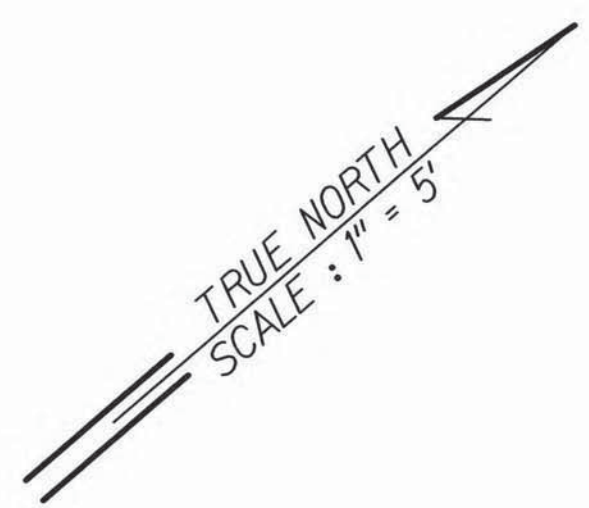


FED. ROAD DIST. NO.	STATE	PROJ. NO.	FISCAL YEAR	SHEET NO.	TOTAL SHEETS
HAWAII	HAW.	61D-01-23	2024	ADD.30	87

**LEGEND**

-  Concrete area
-  Pedestrian detectable warning

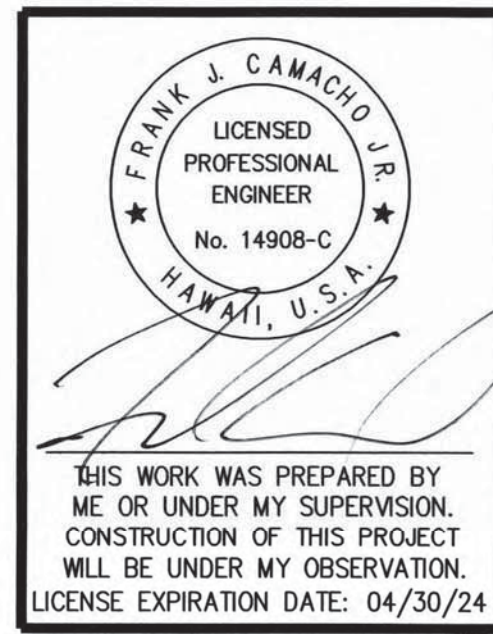
CURVE DATA	
CURVE	①
Δ	73°23'04" $\theta_1 = 18°02'06"$
Δ/2	36°41'32" $\theta_2 = 42°42'23"$
R	26.4' $\theta_3 = 12°39'15"$
T	19.67'
C	31.55'
L	33.81'



**KAILUA ROAD AND ULUOA STREET INTERSECTION  
NORTHWEST SIDEWALK PLAN**

APPROVED: \_\_\_\_\_ DATE: \_\_\_\_\_  
CHIEF, CIVIL ENGINEERING BRANCH, D.P.P.  
 (FOR CONSTRUCTION IN CITY RIGHT-OF-WAY ONLY)

1/11/24  Revised Curb Ramps  
 DATE REVISION



**CURB RAMP DETAILS - 5**

KAILUA ROAD INTERSECTION IMPROVEMENTS  
 Vicinity of Ulua Street and Ulumanu Drive  
 Project No. 61D-01-23

Scale: 1" = 5' Date: DEC, 2023




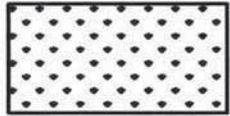

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SURVEY PLOTTED BY	DATE
DRAWN BY	
DESIGNED BY	
QUANTITIES BY	
CHECKED BY	
ORIGINAL PLAN	
NOTE BOOK	
No.	



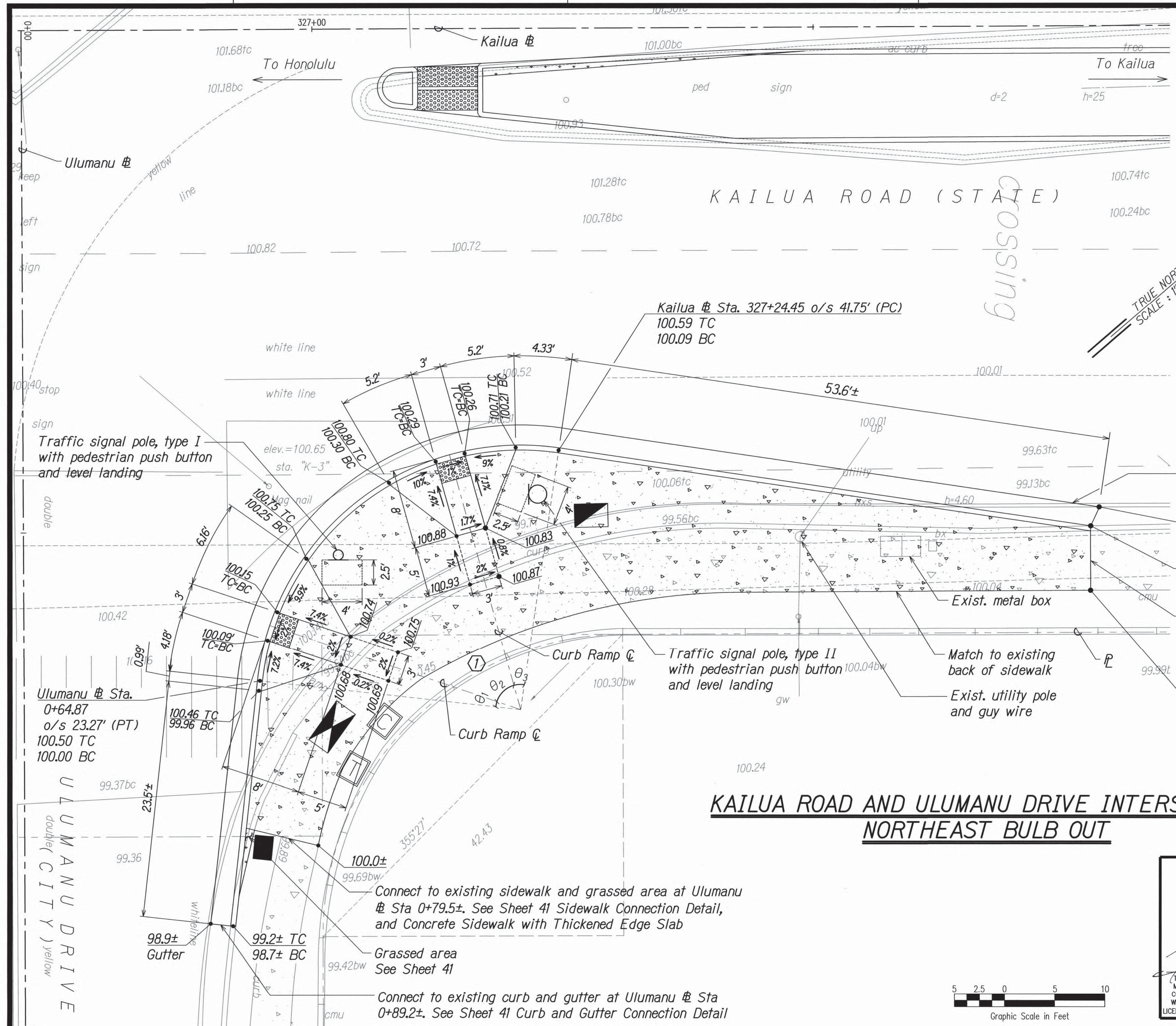
FED. ROAD DIST. NO.	STATE	PROJ. NO.	FISCAL YEAR	SHEET NO.	TOTAL SHEETS
HAWAII	HAW.	61D-01-23	2024	ADD.33	87

### LEGEND

-  Concrete area
-  Grassed area
-  Pedestrian detectable warning

CURVE DATA	
CURVE	①
Δ	83°30'36"
Δ/2	41°45'18"
R	26.4'
T	23.57'
Ch	35.16'
L	38.48'

$\theta_1 = 12^\circ 19' 48''$   
 $\theta_2 = 55^\circ 49' 53''$   
 $\theta_3 = 23^\circ 56' 32''$



## KAILUA ROAD AND ULUMANU DRIVE INTERSECTION NORTHEAST BULB OUT

APPROVED: \_\_\_\_\_ DATE \_\_\_\_\_  
CHIEF, CIVIL ENGINEERING BRANCH, D.P.P. (FOR CONSTRUCTION IN CITY RIGHT-OF-WAY ONLY)

11/11/24  Revised Curb Ramps  
 DATE REVISION

FRANK J. CAMACHO, JR.  
 LICENSED PROFESSIONAL ENGINEER  
 No. 14908-C  
 HAWAII, U.S.A.

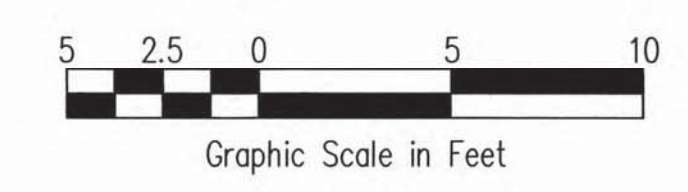
THIS WORK WAS PREPARED BY ME OR UNDER MY SUPERVISION. CONSTRUCTION OF THIS PROJECT WILL BE UNDER MY OBSERVATION. LICENSE EXPIRATION DATE: 04/30/24

STATE OF HAWAII  
 DEPARTMENT OF TRANSPORTATION  
 HIGHWAYS DIVISION

### CURB RAMP DETAILS - 8

KAILUA ROAD INTERSECTION IMPROVEMENTS  
 Vicinity of Ulua Street and Ulumanu Drive  
 Project No. 61D-01-23

Scale: 1" = 5' Date: DEC, 2023



ORIGINAL PLAN	DATE
REVISED BY	DATE
TRACED BY	DATE
DESIGNED BY	DATE
QUANTITIES BY	DATE
CHECKED BY	DATE

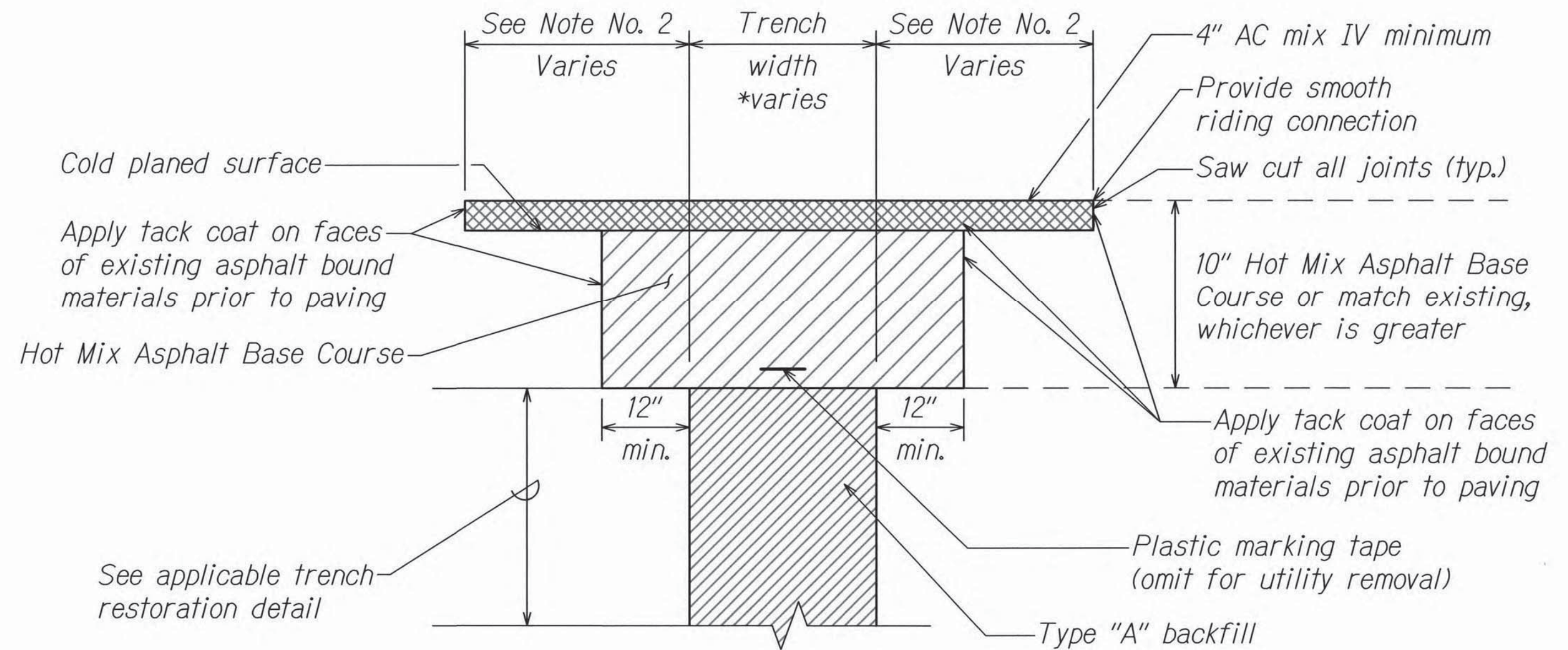
P:\Land Projects\DOT IDIQ - 2022\Traffic Operations Task\PA02 Kailua TrailSig PSE\Drawings\33 Curb Ramp Details - 8.dwg, 1/4/2024, 4:20:29 PM



FED. ROAD DIST. NO.	STATE	PROJ. NO.	FISCAL YEAR	SHEET NO.	TOTAL SHEETS
HAWAII	HAW.	61D-01-23	2024	ADD.39	87

## NOTES

1. Tack coat faces of existing asphalt bound materials prior to filling excavation with new asphalt bound materials.
2. For trench restoration on State-owned Right of Way:
  - a. If the trench is aligned perpendicular to the direction of travel or skewed at angles of 45 degrees or greater to longitudinal direction of the roadway, repave 6 feet on each side of the trench.
  - b. If the trench is aligned parallel to the direction of travel or skewed at angle of less than 45 degrees to the longitudinal direction of roadway, repave to the edge of lane in which edge of trench is located. Repave to the edge of gutter if less than 2-ft from the edge of trench to the edge of gutter.
3. All work performed shall be subject to inspection by the State and shall be to the State's satisfaction.
4. Construct the trench restoration in accordance with the Hawaii Standard Specifications for Road and Bridge Construction (2005) and its Special Provisions, and the Specification of Installation of Miscellaneous Improvements within State Highways.
5. Pavement Smoothness for HDOT roadways - Applicable to areas where the trench activities require shoring, sheet piling and dewatering or as directed by the District Engineer.
  - a. Obtain a profile of the existing roadway that is to have a new surface as a result of the restoration of the trench excavation and submit the profile to the District Engineer before any work for trench excavation begins.
  - b. Obtain a profile of the roadway surface after the roadway surface has been repaved and submit the profile to the District Engineer. The profile of the roadway surface after repaving shall be equal to or smoother than the profile obtained before trench excavation began.
  - c. The distance from the paved surface to the testing edge of a ten-foot long straight edge between two points of contact shall not exceed 3/16 inch.
6. Place all unbound materials in the trench as follows:
  - a. Compaction by water jetting or ponding is not permitted.
  - b. All unbound materials, except the permeable base and ASTM C-33 Size 67:
    - Place material in accordance with Sections 204, 206, 603, 624 and 625 of the Hawaii Standard Specifications for Road and Bridge Construction (2005).
    - Take one compaction test per lift for 300 lineal feet of trench. Submit compaction test results to the District Engineer.
  - c. Permeable Base
    - Place permeable material in uniform horizontal layer not exceeding 9 inches in compacted thickness.
    - Compact each layer with 8 passes of a vibrating plate compactor. Use hand tamper if trench is too narrow to accommodate the vibrating plate compactor.
    - If an existing layer of permeable base is encountered, provide new permeable base to match the existing permeable base thickness and depth and provide a geotextile permeable separator.
  - d. ASTM C-33 Size 67:
    - Material placed under water need not be compacted.
    - Material placed above water:



## TYPICAL TRENCH PAVEMENT SECTION (STATE/CITY)

Not to Scale

1  
40/39

APPROVED:

CHEF, CIVIL ENGINEERING BRANCH, D.P.P.  
(FOR CONSTRUCTION IN CITY RIGHT-OF-WAY ONLY) DATE

1/11/24 DATE △ Revised Detail Name REVISION

	STATE OF HAWAII DEPARTMENT OF TRANSPORTATION HIGHWAYS DIVISION
	<b>PAVING NOTES &amp; DETAILS</b>
THIS WORK WAS PREPARED BY ME OR UNDER MY SUPERVISION. CONSTRUCTION OF THIS PROJECT WILL BE UNDER MY OBSERVATION. LICENSE EXPIRATION DATE: 04/30/24	<b>KAILUA ROAD INTERSECTION IMPROVEMENTS</b> <i>Vicinity of Ulua Street and Ulumanu Drive</i> <b>Project No. 61D-01-23</b> Scale: N/A Date: DEC, 2023

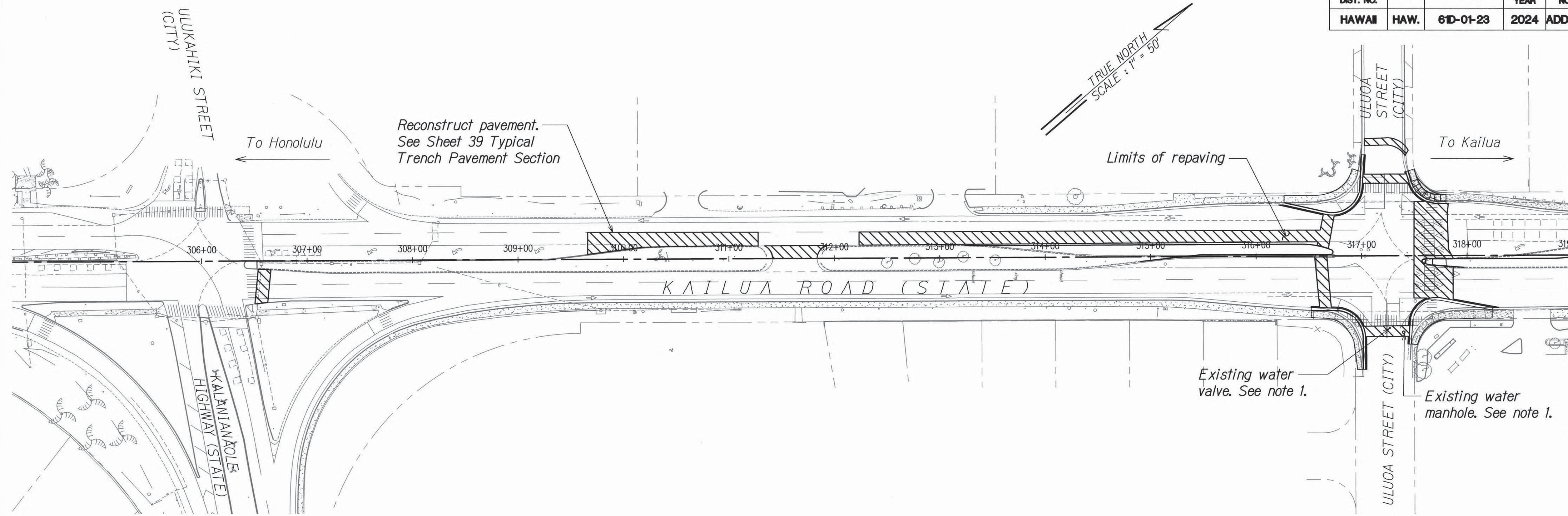
SHEET No. 1 OF 1 SHEETS

ADD. 39

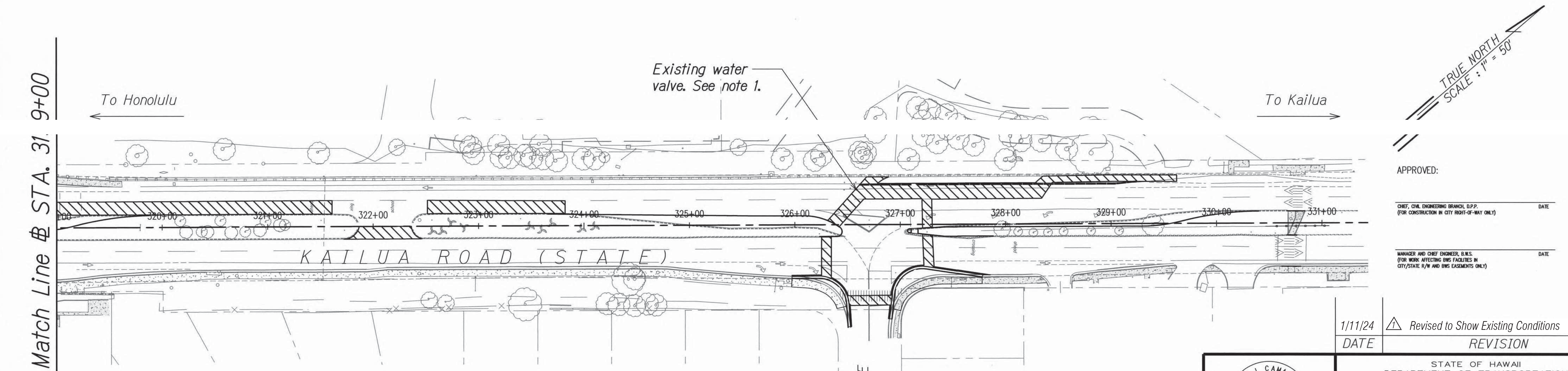
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CHECKED BY	
IN CHARGE	
PROJECT NO.	
DATE	
BY	
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BY	
DATE	



FED. ROAD DIST. NO.	STATE	PROJ. NO.	FISCAL YEAR	SHEET NO.	TOTAL SHEETS
HAWAII	HAW.	61D-01-23	2024	ADD.40	87



Match Line @ STA. 319+00



Match Line @ STA. 319+00

APPROVED:

\_\_\_\_\_  
CHEF, CIVIL ENGINEERING BRANCH, D.P.P.  
(FOR CONSTRUCTION IN CITY RIGHT-OF-WAY ONLY)      DATE

\_\_\_\_\_  
MANAGER AND CHIEF ENGINEER, B.S.  
(FOR WORK AFFECTING BMS FACILITIES IN  
CITY/STATE R/W AND BMS CASSEMENTS ONLY)      DATE

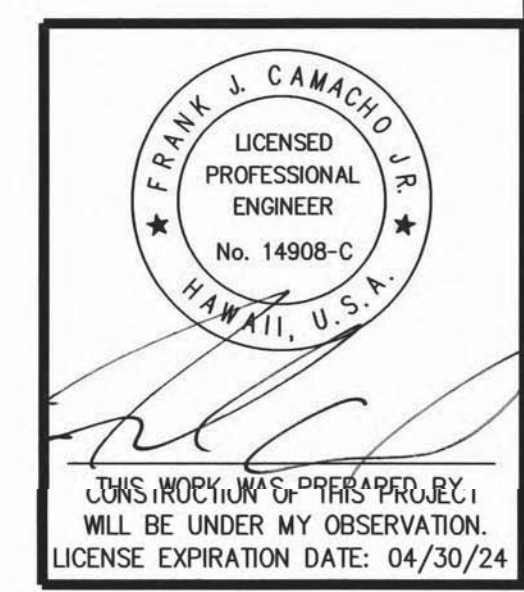
1/11/24 Revised to Show Existing Conditions  
DATE REVISION

STATE OF HAWAII  
DEPARTMENT OF TRANSPORTATION  
HIGHWAYS DIVISION

**PAVING PLAN**

**KAILUA ROAD INTERSECTION IMPROVEMENTS**  
*Vicinity of Ulua Street and Ulumanu Drive*

Scale: 1" = 50'      Date: DEC, 2023



- NOTES:**
- The contractor shall adjust all manhole frames, valve boxes, and meter boxes within the reconstructed or resurfaced areas. The Contractor shall be responsible for and meter boxes to facilitate the adjustments

**LEGEND**

Repaving

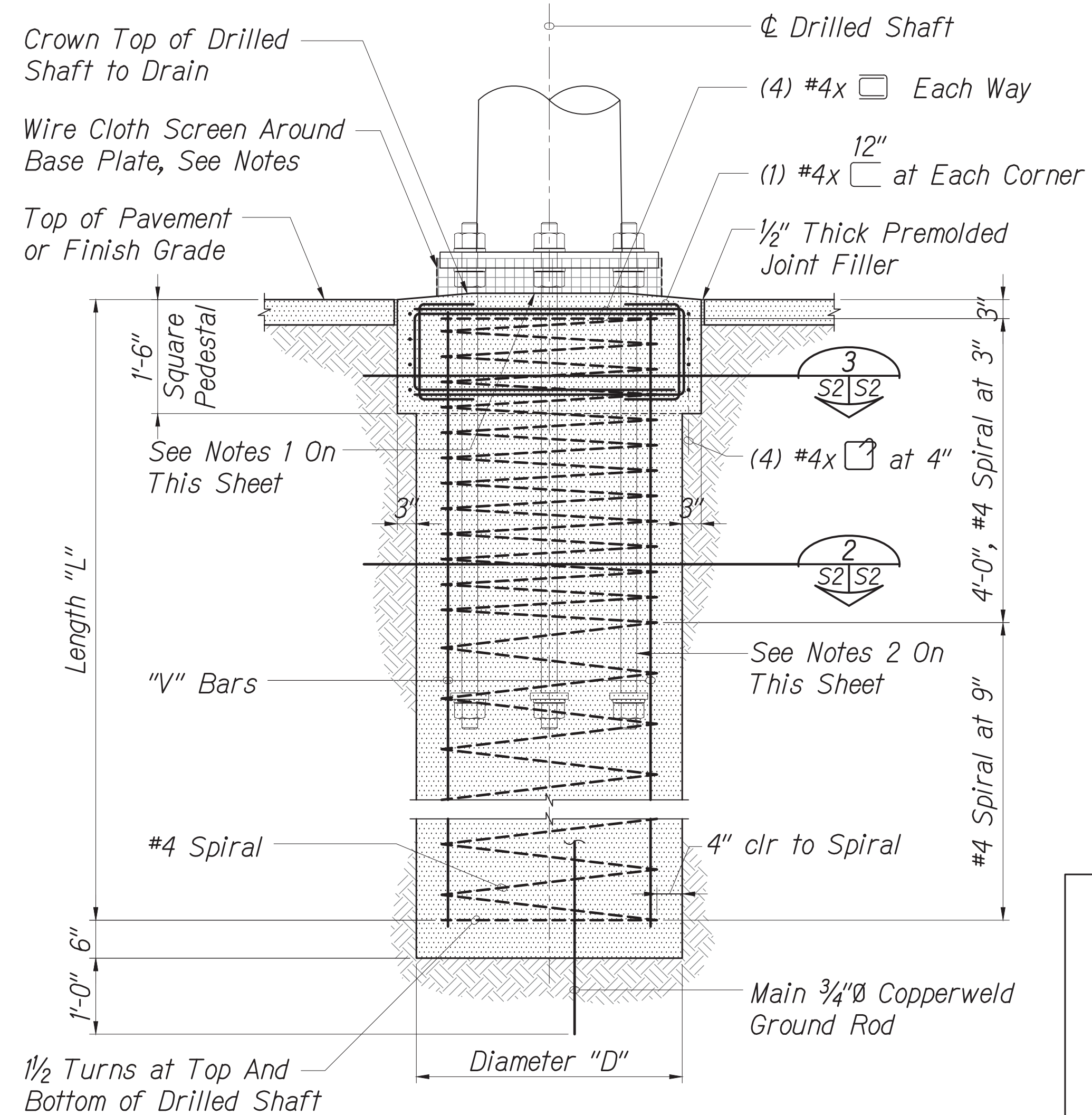


REVISION	DATE

P:\Land Project\TIDQ - 2022\Traffic Operations Task\PAQ 2 Kailua TrafSig PSE\Drawings\40 Paving Plan.dwg, 1/4/2024 4:20:42 PM



FED. ROAD DIST. NO.	STATE	PROJ. NO.	FISCAL YEAR	SHEET NO.	TOTAL SHEETS
HAWAII	HAW.	61D-01-23	2023	ADD.66	87



**Construction Tolerance Note:**  
Anchor Bolts Shall be Installed With Misalignments of Less Than 1:40 From Vertical. After Installation, Firm Contact Shall Exist Between The Anchor Bolt Nuts, Washers, And Base Plate On Any Anchor Bolt Installed In a Misaligned Position.

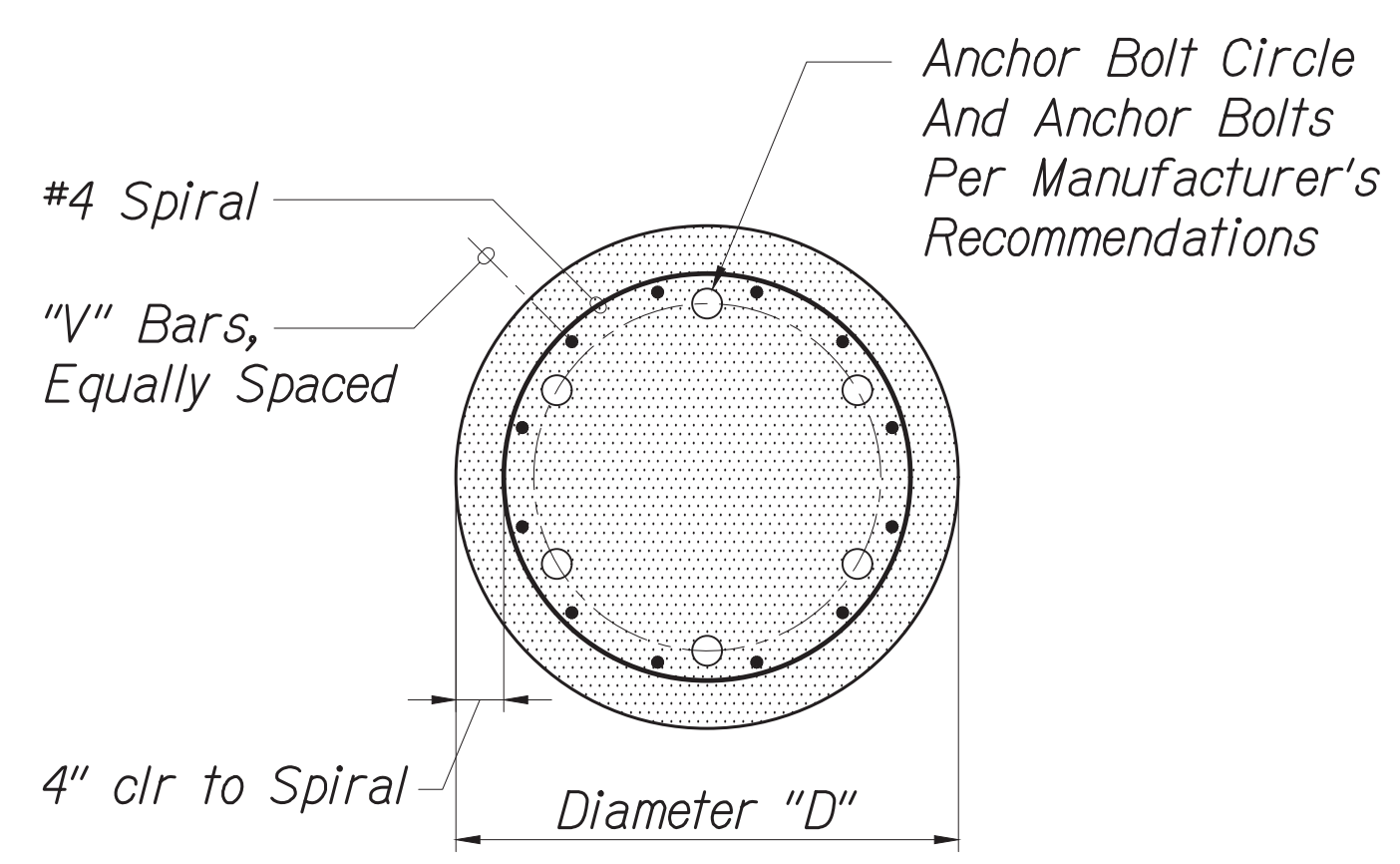
**Wire Cloth Screen Notes:**  
a. A Wire Cloth Screen Shall be Placed Vertically Between The Base Plate And Top of Foundation.  
b. Wire Cloth Shall be Wrapped Horizontally Around The Base Plate With a 3" Minimum Lap.  
c. Wire Cloth Shall be Galvanized Steel, Standard Grade, Plain Weave, 2x2 Mesh, With 0.063" Diameter Wires.  
d. Screen Shall be Attached to The Base Plate With Stainless Steel Self-tapping 1/4" Diameter Screws With Stainless Steel Washers Spaced at 9".

Pole Description	Mast Arm Length	Length "L"	Diameter "D"	Pedestal "P"	"V" Bars
Type II	≤ 30'-0"	12'-0"	3'-6"	4'-0"	(12) #8

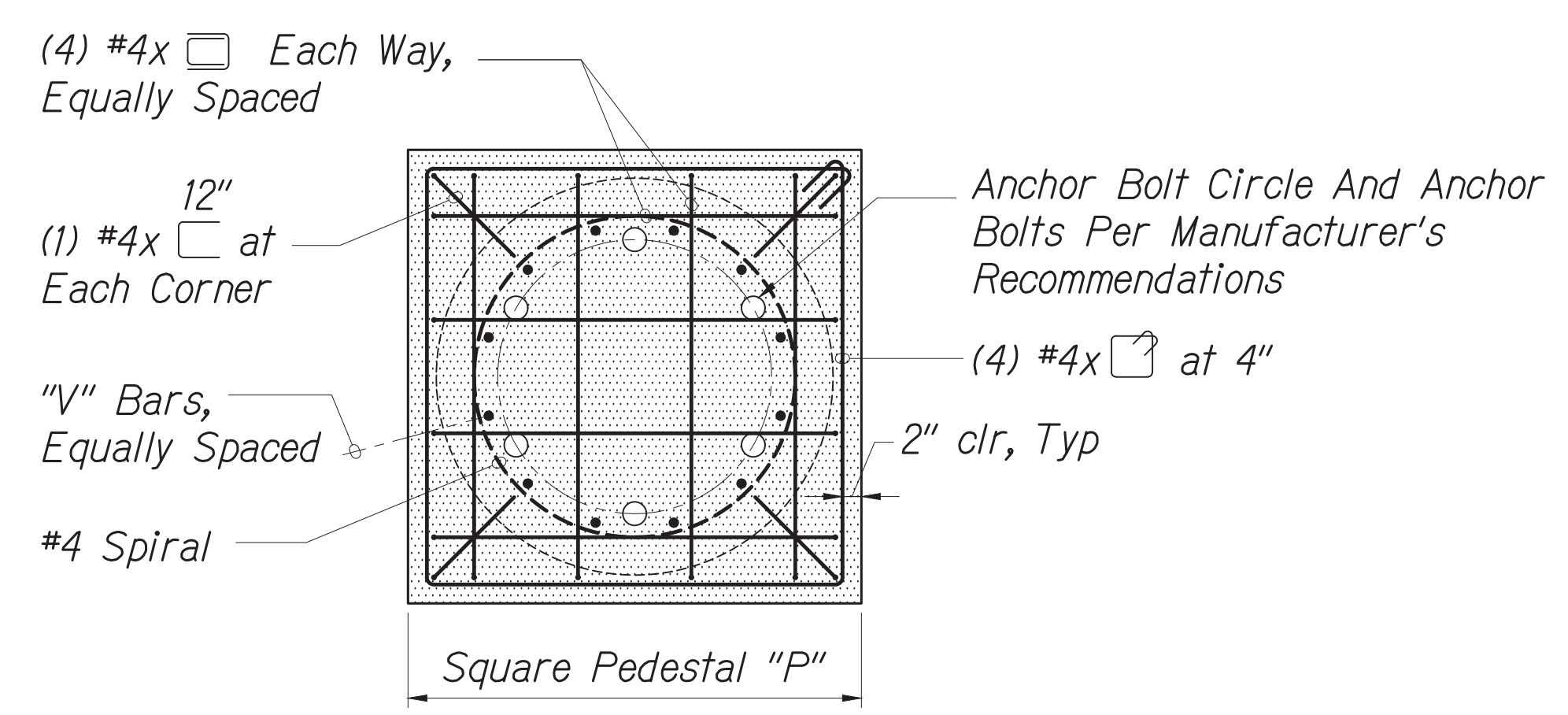
**Notes:**  
1. The Exposed Length of The Anchor Bolt Between The Bottom of The Leveling Nut And The Top of The Drilled Shaft Shall Not Exceed The Anchor Bolt Diameter, Typ  
2. For Base Plate, Bolt Circle Diameter, And Size, Location, Bolt Projection Length, And Length of Anchor Bolts, Follow Manufacturer's Recommendations

**TYPE II TRAFFIC SIGNAL FOUNDATION** 1  
Scale: 3/4" = 1'-0"

SURVEY PLOTTED BY	DATE
DRAWN BY	
TRACED BY	
DESIGNED BY	
NOTE BOOK	
QUANTITIES BY	
CHECKED BY	

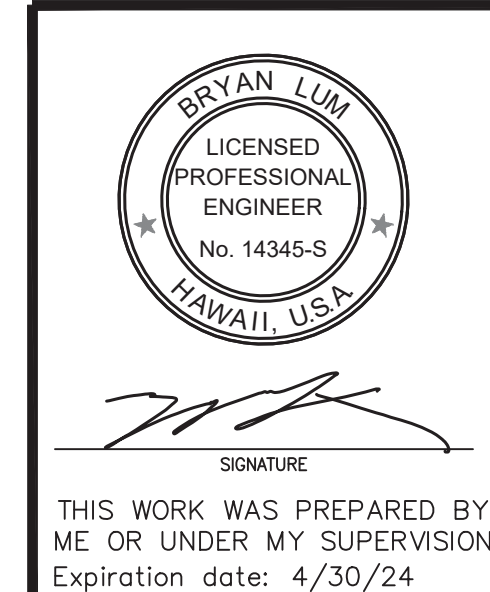


**SECTION** 2  
Scale: 3/4" = 1'-0"



**SECTION** 3  
Scale: 3/4" = 1'-0"

1/11/24	△ Revised Foundation Schedule
DATE	REVISION



STATE OF HAWAII  
DEPARTMENT OF TRANSPORTATION  
HIGHWAYS DIVISION

**DRILLED SHAFT FOUNDATION DETAILS**

KAILUA ROAD INTERSECTION IMPROVEMENTS  
Vicinity of Uluoa Street and Ulumanu Drive  
Project No. 61D-01-23

Scale: As Shown Date: DEC, 2023

P:\6151-62006\6155 C & C of Hon - Traffic Signal Improvements on Kailua Road in the Vicinity of Uluoa St & Ulumanu.Dr050 Drawings\Structural\AutoCAD\_format\2023-11-21\6155\_S-X.dwg, 1/8/2024 1:45:08 PM







FED. ROAD DIST. NO.	STATE	PROJ. NO.	FISCAL YEAR	SHEET NO.	TOTAL SHEETS
HAWAII	HAW.	61D-01-23	2024	ADD.67	87

# GEOTECHNICAL NOTES:

- A geotechnical engineering technical memorandum entitled "Kailua Road Intersection Improvements Vicinity of Ulua Street and Ulumanu Drive" dated December 1st, 2023 has been prepared by Kokua Geotechnical LLC. A copy of the report is on file at the office of the Engineer for review by the Contractor.
- For boring locations, see Sheets 7 & 8 of the report.
- The information presented in the logs of borings depict the subsurface conditions encountered at that specified location and at the time of the field exploration only. Variations of subsoil conditions from those depicted in the logs of borings may occur between and beyond the borings.
- The penetration resistance shown on the logs of borings indicate the number of blows required for the specific sampler type used. The blow counts may need to be factored to obtain the Standard Penetration Test (SPT) blow counts.
- The data given is for general information only. Bidders shall examine the site and the boring data and draw their own conclusions therefrom as to the character of materials to be encountered. The Engineer will not assume responsibility for variations of subsoil quality or conditions other than at the boring locations shown and at the time the borings were taken.

Project: Kailua Road Intersection Improvements Project Location: Kailua, Oahu, Hawaii Project Number: 110922-40		Kokua Geotech LLC 94-974 Pakela Street, Suite 109 Waipahu, HI 96797 (808) 397-6974		Key to Log of Borings Sheet 1 of 1	
---	--	---	--	---------------------------------------	--

1	2	3	4	5	6	7	8	9	10	11	12
Elevation (feet)	Depth (feet)	Sample Type	Sample Number	Sampling Resistance, blow/ft	U.S.C.S.	Graphic Log	MATERIAL DESCRIPTION	Pocket Pen./Torvane, tsf	Water Content, %	Dry Unit Weight, pcf	Remarks and Other Tests

**COLUMN DESCRIPTIONS**

1 Elevation (feet): Elevation (MSL, feet).  
2 Depth (feet): Depth in feet below the ground surface.  
3 Sample Type: Type of soil sample collected at the depth interval shown.  
4 Sample Number: Sample identification number.  
5 Sampling Resistance, blow/ft: Number of blows to advance driven sampler one foot (or distance shown) beyond seating interval using the hammer identified on the boring log.  
6 U.S.C.S.: Type of material encountered.  
7 Graphic Log: Graphic depiction of the subsurface material encountered.  
8 MATERIAL DESCRIPTION: Description of material encountered. May include consistency, moisture, color, and other descriptive text.  
9 Pocket Pen./Torvane, tsf: The reading from Pocket Penetrometer or Torvane.  
10 Water Content, %: Water content of the soil sample, expressed as percentage of dry weight of sample.  
11 Dry Unit Weight, pcf: Dry weight per unit volume of soil sample measured in laboratory, in pounds per cubic foot.  
12 Remarks and Other Tests: Other Tests

**FIELD AND LABORATORY TEST ABBREVIATIONS**

CHEM: Chemical tests to assess corrosivity  
COMP: Compaction test  
CONS: One-dimensional consolidation test  
LL: Liquid Limit, percent  
PI: Plasticity Index, percent  
SA: Sieve analysis (percent passing No. 200 Sieve)  
UC: Unconfined compressive strength test, Qu, in ksf  
WA: Wash sieve (percent passing No. 200 Sieve)

**MATERIAL GRAPHIC SYMBOLS**

Asphaltic Concrete (AC)  
Basalt Rock Formation  
Boulders  
Fat CLAY, CLAY w/SAND, SANDY CLAY (CH)  
Silty GRAVEL (GM)  
Silty SAND (SM)

**TYPICAL SAMPLER GRAPHIC SYMBOLS**

Auger sampler  
Bulk Sample  
3-inch-OD California w/ brass rings  
CME Sampler  
Grab Sample  
HQ Coring  
3-inch OD Modified California w/ brass liners  
Pitcher Sample  
PQ Coring  
Probing w/Pointed Tip  
2-inch-OD unlined split spoon (SPT)  
Shelby Tube (Thin-walled, fixed head)

**OTHER GRAPHIC SYMBOLS**

Water level (at time of drilling, ATD)  
Water level (after washing)  
Minor change in material properties within a stratum  
Inferred/gradational contact between strata  
Queried contact between strata

**GENERAL NOTES**

1: Soil classifications are based on the Unified Soil Classification System. Descriptions and stratum lines are interpretive, and actual lithologic changes may be gradual. Field descriptions may have been modified to reflect results of lab tests.  
2: Descriptions on these logs apply only at the specific boring locations and at the time the borings were advanced. They are not warranted to be representative of subsurface conditions at other locations or times.

PLATE A-0.1

Project: Kailua Road Intersection Improvements Project Location: Kailua, Oahu, Hawaii Project Number: 110922-40		Kokua Geotech LLC 94-974 Pakela Street, Suite 109 Waipahu, HI 96797 (808) 397-6974		Log of Boring No. 1 Sheet 1 of 1	
---	--	---	--	-------------------------------------	--

Date(s) Drilled: 11/27/23	Logged By: CH	Checked By: AJF
Drilling Method: CF Auger	Drill Bit Size/Type: 4-inch Solid Stem Auger	Total Depth of Borehole: 21.5 feet
Drill Rig Type: Mobile B-53	Drilling Contractor: Kokua Geotech LLC	Approximate Surface Elevation: +103 feet MSL*
Groundwater Level and Date Measured: Not Encountered	Sampling Method(s): MCS & SPT	Hammer Data: 140 lbs. with 30-inch drop
Borehole Backfill: Soil Cuttings, Gravel, and AC Patch	Location: See Site Plan (Plate 2.1)	

1	2	3	4	5	6	7	8	9	10	11	12
Elevation (feet)	Depth (feet)	Sample Type	Sample Number	Sampling Resistance, blow/ft	U.S.C.S.	Graphic Log	MATERIAL DESCRIPTION	Pocket Pen./Torvane, tsf	Water Content, %	Dry Unit Weight, pcf	Remarks and Other Tests

103 0  
1 35  
2 10  
5 19  
10 11  
15 29  
20 10  
25 25

8-inch ASPHALTIC CONCRETE  
Grayish brown SILTY GRAVEL with some sand, medium dense, moist (base material)  
Reddish brown SILTY CLAY with some sand and gravel (coralline), stiff, moist (fill)  
Reddish brown to brown SILTY CLAY with some sand and a little gravel, stiff, moist (alluvium)  
grades to very stiff  
Reddish brown with multi-color mottling SILTY CLAY with some sand and decomposed gravel, very stiff, moist (alluvium)  
grades to stiff  
Boring terminated at approximately 21.5 feet below the existing ground surface  
\*Elevations of borings estimated from Google Earth imagery

PLATE A-1

Project: Kailua Road Intersection Improvements Project Location: Kailua, Oahu, Hawaii Project Number: 110922-40		Kokua Geotech LLC 94-974 Pakela Street, Suite 109 Waipahu, HI 96797 (808) 397-6974		Log of Boring No. 2 Sheet 1 of 1	
---	--	---	--	-------------------------------------	--

Date(s) Drilled: 11/27/23	Logged By: CH	Checked By: AJF
Drilling Method: CF Auger	Drill Bit Size/Type: 4-inch Solid Stem Auger	Total Depth of Borehole: 8.5 feet
Drill Rig Type: Mobile B-53	Drilling Contractor: Kokua Geotech LLC	Approximate Surface Elevation: +102 feet MSL*
Groundwater Level and Date Measured: Not Encountered	Sampling Method(s): MCS & SPT	Hammer Data: 140 lbs. with 30-inch drop
Borehole Backfill: Soil Cuttings, Gravel, and AC Patch	Location: See Site Plan (Plate 2.1)	

1	2	3	4	5	6	7	8	9	10	11	12
Elevation (feet)	Depth (feet)	Sample Type	Sample Number	Sampling Resistance, blow/ft	U.S.C.S.	Graphic Log	MATERIAL DESCRIPTION	Pocket Pen./Torvane, tsf	Water Content, %	Dry Unit Weight, pcf	Remarks and Other Tests

102 0  
1 30"  
2 20"  
5 12  
20" Ref.  
25 25

8-inch ASPHALTIC CONCRETE  
Grayish brown SILTY GRAVEL with some sand, medium dense, moist (base material)  
Light brown SILTY CLAY with some sand and gravel, stiff, moist (fill)  
Gray BOULDER, hard (alluvium)  
Brown SILTY CLAY with some sand and gravel, hard, moist (alluvium)  
Gray BOULDER, hard (alluvium)  
Boring terminated at approximately 7.0 feet below the existing ground surface on an apparent hard boulder

PLATE A-2

ORIGINAL PLAN	DATE
SURVEY PLOTTED BY	
DRAWN BY	
TRACED BY	
DESIGNED BY	
QUANTITIES BY	
CHECKED BY	

11/11/24  
DATE  
Replaced Boring Logs  
REVISION

STATE OF HAWAII  
DEPARTMENT OF TRANSPORTATION  
HIGHWAYS DIVISION

**BORING LOG - 1**

KAILUA ROAD INTERSECTION IMPROVEMENTS  
Vicinity of Ulua Street and Ulumanu Drive  
Project No. 61D-01-23

Scale: N/A Date: DEC, 2023

SHEET No. 1 OF 2 SHEETS



FED. ROAD DIST. NO.	STATE	PROJ. NO.	FISCAL YEAR	SHEET NO.	TOTAL SHEETS
HAWAII	HAW.	61D-01-23	2024	ADD.68	87

## GEOTECHNICAL NOTES:

- A geotechnical engineering technical memorandum entitled "Kailua Road Intersection Improvements Vicinity of Uluoa Street and Ulumanu Drive" dated December 1st, 2023 has been prepared by Kokua Geotechnical LLC. A copy of the report is on file at the office of the Engineer for review by the Contractor.
- For boring locations, see Sheets 7 & 8 of the report.
- The information presented in the logs of borings depict the subsurface conditions encountered at that specified location and at the time of the field exploration only. Variations of subsoil conditions from those depicted in the logs of borings may occur between and beyond the borings.
- The penetration resistance shown on the logs of borings indicate the number of blows required for the specific sampler type used. The blow counts may need to be factored to obtain the Standard Penetration Test (SPT) blow counts.
- The data given is for general information only. Bidders shall examine the site and the boring data and draw their own conclusions therefrom as to the character of materials to be encountered. The Engineer will not assume responsibility for variations of subsoil quality or conditions other than at the boring locations shown and at the time the borings were taken.

Project: Kailua Road Intersection Improvements Project Location: Kailua, Oahu, Hawaii Project Number: 110922-40	<b>Kokua Geotech LLC</b> 94-974 Pakela Street, Suite 109 Waipahu, HI 96797 (808) 397-6974	Log of Boring No. 3 Sheet 1 of 1
Date(s) Drilled: 11/28/23	Logged By: CH	Checked By: AJF
Drilling Method: CF Auger	Drill Bit Size/Type: 4-inch Solid Stem Auger	Total Depth of Borehole: 13.5 feet
Drill Rig Type: Mobile B-53	Drilling Contractor: Kokua Geotech LLC	Approximate Surface Elevation: +88 feet MSL*
Groundwater Level and Date Measured: Not Encountered	Sampling Method(s): MCS & SPT	Hammer Data: 140 lbs. with 30-inch drop
Borehole Backfill: Soil Cuttings, Gravel, and AC Patch	Location: See Site Plan (Plate 2.2)	

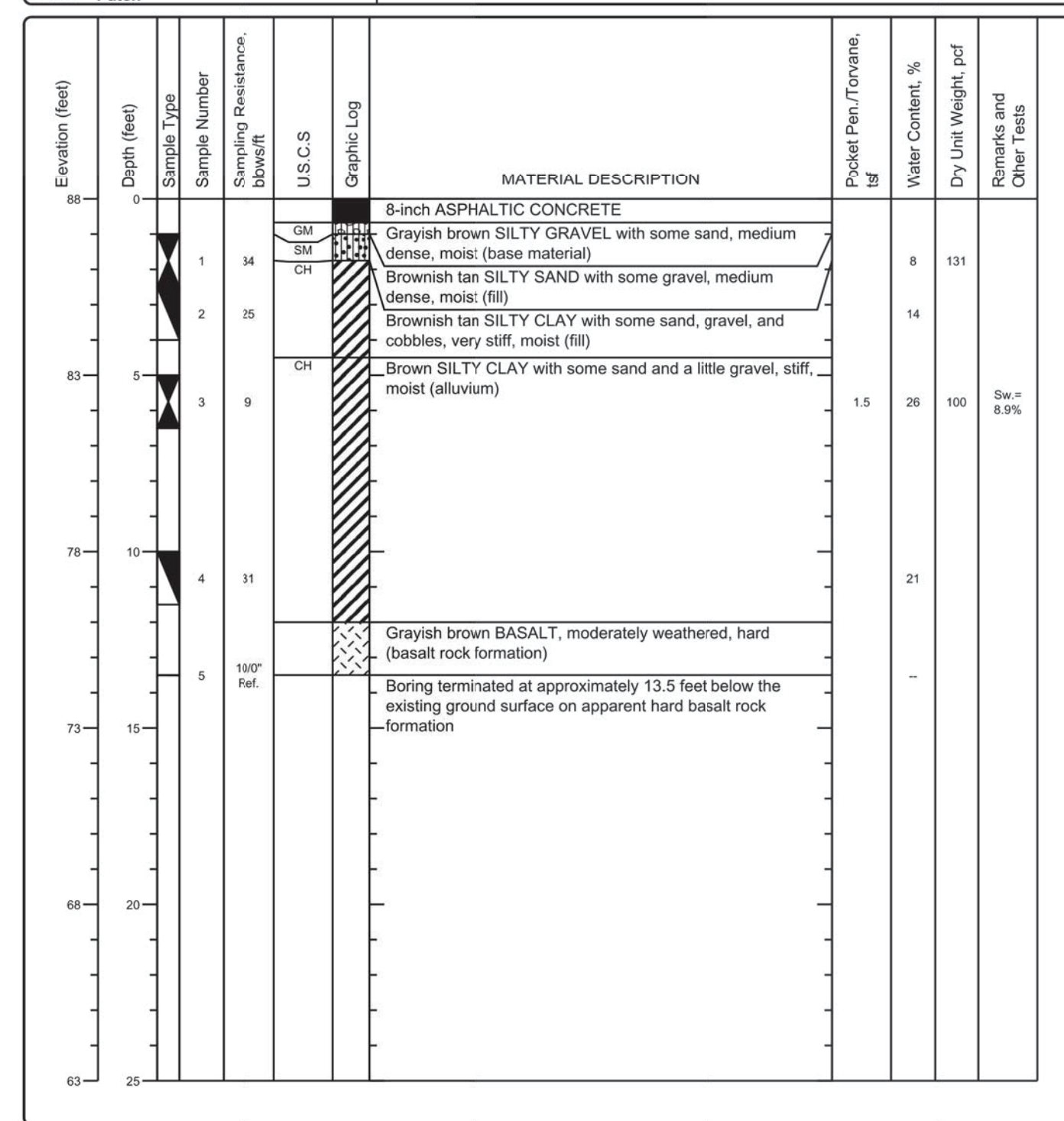


PLATE A-3

Project: Kailua Road Intersection Improvements Project Location: Kailua, Oahu, Hawaii Project Number: 110922-40	<b>Kokua Geotech LLC</b> 94-974 Pakela Street, Suite 109 Waipahu, HI 96797 (808) 397-6974	Log of Boring No. 4 Sheet 1 of 1
Date(s) Drilled: 11/28/23	Logged By: CH	Checked By: AJF
Drilling Method: CF Auger	Drill Bit Size/Type: 4-inch Solid Stem Auger	Total Depth of Borehole: 15.3 feet
Drill Rig Type: Mobile B-53	Drilling Contractor: Kokua Geotech LLC	Approximate Surface Elevation: +91 feet MSL*
Groundwater Level and Date Measured: Not Encountered	Sampling Method(s): MCS & SPT	Hammer Data: 140 lbs. with 30-inch drop
Borehole Backfill: Soil Cuttings, Gravel, and AC Patch	Location: See Site Plan (Plate 2.2)	

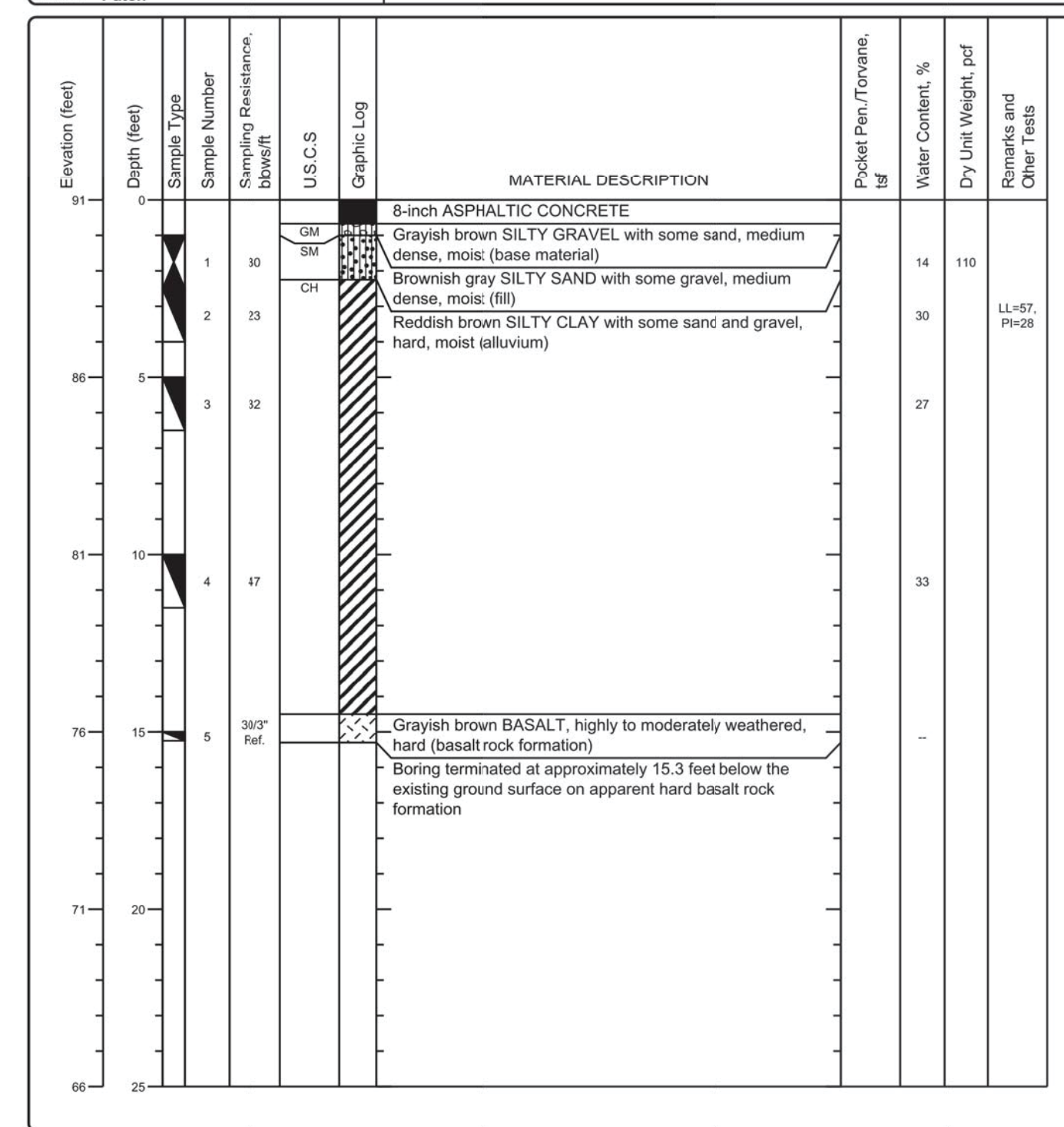


PLATE A-4

ORIGINAL PLAN	DATE
SURVEY PLOTTED BY	
DRAWN BY	
TRACED BY	
DESIGNED BY	
QUANTITIES BY	
CHECKED BY	

1/11/24 Replaced Boring Logs  
DATE REVISION

STATE OF HAWAII  
DEPARTMENT OF TRANSPORTATION  
HIGHWAYS DIVISION

**BORING LOG - 2**

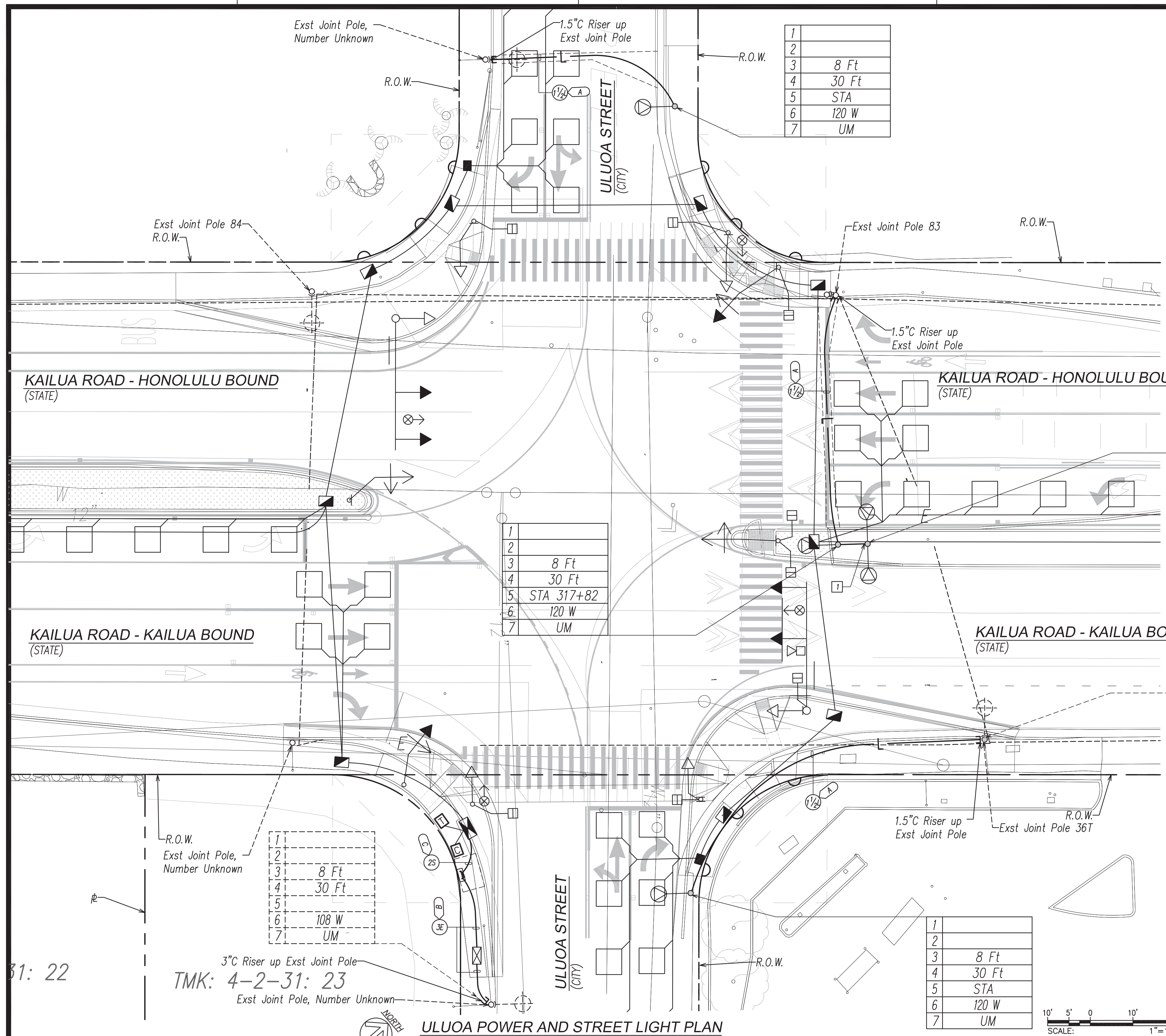
**KAILUA ROAD INTERSECTION IMPROVEMENTS**  
**Vicinity of Uluoa Street and Ulumanu Drive**  
**Project No. 61D-01-23**

Scale: N/A Date: DEC, 2023

SHEET No. 2 OF 2 SHEETS



FED. ROAD DIST. NO.	STATE	PROJ. NO.	FISCAL YEAR	SHEET NO.	TOTAL SHEETS
HAWAII	HAW.	61D-01-23	2024	ADD. 81	87



1	
2	
3	8 Ft
4	30 Ft
5	STA
6	120 W
7	UM

1	
2	
3	8 Ft
4	30 Ft
5	STA 317+82
6	120 W
7	UM

1	
2	
3	8 Ft
4	30 Ft
5	STA 317+89
6	120 W
7	UM

1	
2	
3	8 Ft
4	30 Ft
5	
6	108 W
7	UM

1	
2	
3	8 Ft
4	30 Ft
5	STA
6	120 W
7	UM

- Notes:
- 1 Provide 1 1/2" C and Wiring Between Light Pole Bases.
  - 2 Provide Select Granular Bedding Beneath Handholes Shall Be Placed In Relatively Uniform Lifts No Greater Than Eight (8) Inches In Loose Thickness, Moisture Conditioned To Optimum Moisture Content, And Uniformly Compacted To At Least Ninety (90) Percent Of The Maximum Dry Density (ASTM D1557-02). Contractor To Schedule With Owner's Geotechnical Inspector For Compaction And Bedding Installation. Pullbox/Handholes To Be Installed Flush With Walkway.

Chief, Civil Engineering Branch  
 Department of Planning & Permitting  
 City & County of Honolulu  
 (FOR CONSTRUCTION IN CITY RIGHT-OF-WAY ONLY)

Program Administrator, Mechanical/Electrical  
 Department of Design and Construction  
 City & County of Honolulu  
 (FOR CONSTRUCTION IN CITY RIGHT-OF-WAY ONLY)

1/11/24	Revised to Show Existing Conditions
DATE	REVISION

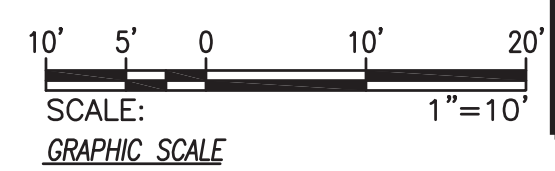
THIS WORK WAS PREPARED BY ME OR UNDER MY SUPERVISION AND CONSTRUCTION OF THIS PROJECT WILL BE UNDER MY OBSERVATION.

STATE OF HAWAII  
 DEPARTMENT OF TRANSPORTATION  
 HIGHWAYS DIVISION

**ULUOA POWER AND STREET LIGHT PLAN**

**KAILUA ROAD INTERSECTION IMPROVEMENTS**  
 Vicinity of Ulua Street and Ulumanu Drive  
 Project No. 61D-01-23

Scale: As Noted      Date: DEC, 2023



Z:\ACAD\PROJECTS\223057\E004 DIST1. 12/29/2023 9:40AM Rsilva

DATE	
SURVEY PLOTTED BY	
ORIGINAL PLAN	
NOTE BOOK	
DESIGNED BY	
QUANTITIES BY	
CHECKED BY	
No.	

31: 22

3" C Riser up Exst Joint Pole  
 TMK: 4-2-31: 23  
 Exst Joint Pole, Number Unknown



**ULUOA POWER AND STREET LIGHT PLAN**  
 SCALE: 1" = 10'

## PRE-BID MEETING MINUTES

Project: Kailua Road Intersection Improvements, vicinity of Ulua Street and Ulumanu Drive  
Project No. 61D-01-23  
District of Koolaupoko  
Island of Oahu

Subject: Non-mandatory Pre-bid Conference

Date/Time: December 28, 2023 / 2:00 PM

Held: Virtually via Microsoft Teams

Present: See attached lists of attendees

Discussed:

A. Reid Tokuhara opens meeting at 2:00 P.M.:

1. Introductions.
2. Pre-bid conference is non-mandatory and is intended for clarification prior to bidding.
3. See attached list of attendees.
4. Any discrepancies will be addressed by addendum.
5. Bidders had until January 2, 2024 at 2:00 P.M. to submit any questions.
6. The minutes and agenda to this meeting will be included in an Addendum.
7. Bid opening is scheduled for January 18, 2024 at 2:00 P.M.
8. Geotechnical Engineering Exploration for Kailua Road Intersection Improvements, Vicinity of Ulua Street and Ulumanu Drive dated December 22, 2023 will be included in an Addendum.
9. Removal of raised crosswalks will be done immediately after the activation of the signals, as also stated in the plans.

B. Open discussion to prospective bidders:

No comments were received from the prospective bidder.

Meeting Adjourned at 2:05 P.M.

Prepared by: Reid Tokuhara

Pre-Bid Meeting Attendance List  
 Kailua Road Intersection Improvements, vicinity of Ulua Street and Ulumanu Drive, Project No. 61D-01-23  
 December 28, 2023 at 2:00pm

NAME	Organization
1 Reid Tokuhara	HDOT - HWY
2 Steven Yoshida	HDOT - HWY
3 Frank Camacho	Community Planning and Engineering, Inc.
4 Juanita Wolfgramm	Community Planning and Engineering, Inc.
5 Joshua Ramelb	Community Planning and Engineering, Inc.
6 Zey Tong	ICX Transportation Group
7 Jason Ames	Grace Pacific LLC
8	
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19	
20	



# **GEOTECHNICAL ENGINEERING EXPLORATION**

**KAILUA ROAD INTERSECTION IMPROVEMENTS  
VICINITY OF ULUOA STREET AND ULUMANU DRIVE  
KAILUA, OAHU, HAWAII**

**DECEMBER 22, 2023**

Prepared for:  
**COMMUNITY PLANNING AND ENGINEERING, INC.**

**PROJECT NO. 110922-00**



**Kokua Geotech LLC**  
Soil and Foundation Engineering

December 22, 2023  
Project No. 110922-00

**Community Planning and Engineering, Inc.**  
1286 Queen Emma Street  
Honolulu, HI 96813

Attention: Mr. Frank J. Camacho

Subject: **Geotechnical Engineering Exploration**  
Kailua Road Intersection Improvements  
Vicinity of Ulua Street and Ulumanu Drive  
Kailua, Oahu, Hawaii

Dear **Mr. Camacho**:

We are pleased to submit this report entitled “Geotechnical Engineering Exploration, Kailua Road Intersection Improvements, Vicinity of Ulua Street and Ulumanu Drive, Kailua, Oahu, Hawaii” prepared for the design of the project.

The purpose of our field exploration and this report was to observe and evaluate the general subsurface conditions at accessible locations at the project site to formulate geotechnical recommendations to assist in the design of the project. Our work was performed in general accordance with the scope of services outlined in our fee proposal dated November 10, 2022.

Our findings and recommendations are summarized as follows:

1. Our field exploration generally encountered pavement structures consisting of about 8 inches of asphaltic concrete and 4 inches of base material overlying surface fill materials, alluvial soils, and hard basalt rock formation extending down to the maximum depth explored of about 21.5 feet below the existing ground surface. The surface fill materials were encountered to depths ranging from about 2 to 5 feet below the existing ground surface and generally consisted of stiff to very stiff silty clay and medium dense silty sand with some gravel.

Alluvial soils were encountered underlying the surface fill materials to depths ranging from about 12 feet to the maximum depth explored of about 21.5 feet below the existing ground surface in Boring No. 1 and generally consisted of stiff to hard silty clay with some sand and gravel. In addition, boulders were encountered within the alluvial soil deposits in Boring No. 2 at depths of about 2 and 6 feet below the existing ground surface.



Hard basalt rock formation was encountered underlying the alluvial soils in Boring Nos. 3 and 4 only, and extended down to the maximum depth explored in these boring of about 15.3 feet.

2. We did not encounter groundwater in the borings at the time of our field exploration. However, it should be noted that groundwater levels are subject to change due to rainfall, time of year, seasonal precipitation, surface water runoff, and other factors. In addition, subterranean seepage may be encountered during construction due to high rainfall in the area, sloping terrain and relict structure in the alluvial soils and basalt rock formation encountered.
3. Based on the loading demands provided by the project structural engineer and the subsurface soil conditions encountered at the site, we believe the new traffic signal structures may be supported by a deep foundation system consisting of cast-in-place concrete drilled shafts.
4. Based on the bolt circle diameter and the square bearing plate dimensions, we envision that drilled shaft foundations with minimum diameters of 24 and 42 inches may be used to support the new Type 1 and Type 2 traffic signal structures, respectively.
5. In general, we recommend installing 24-inch diameter drilled shaft foundations with minimum embedment lengths of 6 feet below the design finished grade to support the new Type 1 traffic signal structures planned for the project.

In addition, we recommend installing 42-inch diameter drilled shaft foundations with minimum embedment lengths of 12 feet below the design finished grade to support the new Type 2 traffic signal structures planned for the project. The project structural engineer should verify the drilled shaft structural capacity for the calculated induced stresses.

6. It should be noted that cobbles, boulders, and hard basalt rock formation may be encountered within the depth of the drilled shafts. Therefore, some difficult drilling conditions may be expected. The drilled shaft subcontractor should have the appropriate equipment and tools to drill through these types of natural obstructions, where encountered. The drilled shaft subcontractor should demonstrate that the proposed drilling equipment will be capable of installing the drilled shafts to the recommended depths and dimensions.
7. The construction plans and specifications for the project should be forwarded to us for review to determine whether the recommendations contained in this report are adequately reflected in those documents. If this review is not made, Kokua

Geotech LLC cannot assume responsibility for misinterpretation of our recommendations.

8. Kokua Geotech LLC should also be retained to monitor the drilled shaft installation work and other aspects of earthwork construction to determine whether the recommendations of this report are followed. The recommendations presented herein are contingent upon such observations.

If the actual exposed subsurface soil conditions encountered during construction differ from those assumed or considered in this report, Kokua Geotech LLC should be contacted to review and/or revise the geotechnical recommendations presented herein.

Detailed discussion of our findings and geotechnical engineering recommendations are contained in the body of this report. We appreciate the opportunity to be of service for this project. Should you have any questions concerning this report, please contact our office.

Very truly yours,

**Kokua Geotech LLC**



---

**Xiaobin (Tim) Lin, P.E.**  
President

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**GEOTECHNICAL ENGINEERING EXPLORATION  
KAILUA ROAD INTERSECTION IMPROVEMENTS  
VICINITY OF ULUOA STREET AND ULUMANU DRIVE  
KAILUA, OAHU, HAWAII**

---

**SECTION 1.0 INTRODUCTION**

---

We have performed a geotechnical engineering exploration for the *Kailua Road Intersection Improvements* project in Kailua on the Island of Oahu, Hawaii. The location of the project and general vicinity are shown on the Project Location Map, Plate 1.

The purpose of our exploration was to observe and evaluate the general subsurface conditions at accessible locations at the project site to formulate geotechnical recommendations to assist in the design of the project. This report summarizes the findings and presents our geotechnical recommendations resulting from our site reconnaissance, field exploration, laboratory testing, and engineering analyses for the project. The findings and recommendations presented herein are subject to the limitations noted at the end of this report.

**1.1 PROJECT CONSIDERATIONS**

The project generally involves traffic improvements at two intersections along Kailua Road in Kailua on the Island of Oahu, Hawaii. In general, the planned intersection improvements along Kailua Road are at Uluoa Street and Ulumanu Drive and generally involve the installation of new traffic signal systems. Layouts of the project sites are shown on the Site Plans, Plates 2.1 and 2.2.

Based on the information provided, we understand the new traffic signal systems will generally consist of traffic signals, underground ducts, poles, foundations, interconnect, controller hardware and other appurtenant equipment, such as CCTV camera(s). It is our understanding that Type 1 traffic signal structures and Type 2 traffic signal structures with 30-foot mast arms are planned for the intersections. The project structural engineer provided the following preliminary structural loading information for the foundation design analyses of the new traffic signal pole structures.

<b>PRELIMINARY STRUCTURAL LOADING INFORMATION</b>	
<b>Type 1 Traffic Signal Structure</b>	
Axial Load (y-direction)	0.2 kips
Horizontal Load (z-direction)	1 kips
Overturning Moment (z-direction)	6 kip-feet
Torsion (z-direction)	0.7 lb-feet
<b>Type 2 Traffic Signal Structure with 30-foot Mast Arm</b>	
Axial Load (y-direction)	3 kips
Horizontal Load (z-direction)	6 kips
Overturning Moment (z-direction)	22 kip-feet
Overturning Moment (x-direction)	99 kip-feet
Torsion (z-direction)	67 kip-feet

## **1.2 PURPOSE AND SCOPE OF WORK**

The purpose of our services was to generally explore and evaluate the subsurface soil conditions at accessible locations at the project site to provide geotechnical recommendations to assist in the design of the project. The work was performed in general accordance with our fee proposal dated November 10, 2022. The scope of work for this exploration included the following items:

1. Research and review of available in-house soils boring data and other information for the project.
2. Coordination of boring stake-out and utility clearances at the proposed boring locations by our field engineer.
3. Mobilization and demobilization of a truck-mounted drill rig and two operators to the project site and back.
4. Drilling and sampling of four boreholes extending to depths ranging from about 8.5 to 21.5 feet below the existing ground surface.
5. Coordination of the field exploration and logging of the borings by our field engineer.

## SECTION 1.0 INTRODUCTION

---

6. Laboratory testing of selected soil samples obtained during the field exploration as an aid in classifying the materials and evaluating their engineering properties.
7. Analyses of the field and laboratory data to formulate geotechnical recommendations to assist in the design of the project.
8. Preparation of this report summarizing our work on the project and presenting our findings and recommendations.
9. Coordination of our overall work on the project by our senior engineer.
10. Quality assurance and client/design team consultation by our principal engineer.
11. Miscellaneous work efforts such as drafting, word processing, and clerical support.

Detailed descriptions of our field exploration methodology are presented in the following section and the Logs of Borings are presented in Appendix A. Results of the laboratory tests performed are presented in Appendix B.

---

*END OF INTRODUCTION*

---

## SECTION 2.0 SITE CHARACTERIZATION AND FINDINGS

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### 2.1 GENERAL SITE GEOLOGY

The project site is generally located on the southeastern flank of the Koolau Volcano on the Island of Oahu. Based on the geologic maps of the Island of Oahu (Stearns, 1939 and Sherrod and others, 2007), the general area of the project site is underlain by Honolulu Volcanics Training School Lava Flows (Qol) and Older Alluvium (QTao). In general, the rocks associated with Qol are generally characterized by flows of jointed, dense vesicular basalt with interbedded thin clinker layers. In-situ weathering of these lava flows has occurred, forming a mantle of residual and saprolitic soils overlying the top of the basalt rock formation.

In general, saprolite is composed mainly of silty material that may exhibit a relict structure (vesicles, joints, etc.) from its parent rock, while residual soil tends to be more clayey and is usually “structureless.” Both residual and saprolitic soils are typical of the tropical weathering of volcanic rocks. The residual and saprolitic soils grade to basaltic rock formation with increased depth.

Erosional processes in the mountainous regions of Hawaii are dominated by the detachment of soil and rock masses from the valley walls which are transported downslope toward the axis of the valley primarily by gravity as colluvium. Colluvial accumulations often consist of material that is generally deposited by gravity fall, rain wash and mudflow. Once these materials reach the stream in the central portion of the valley, alluvial processes become dominant, and the sediments are transported and deposited as alluvium.

In general, all the big valleys in Hawaii have flat floors built by alluvium deposited by their streams. Stream flows in Hawaii are intermittent and flashy, such that the stream flows transmit large volumes of water for very short duration. Because of this, transport of sediments is intermittent, and the bulk of the stream's hydraulic load consists of a poorly-sorted mixture of boulders, cobbles, gravel, sands, and fines. When the erosional base levels change, these sediment loads are left as deposits.

The surface soils underlying the project site are classified as Pohakupu Silty Clay Loam (PkB) by the U.S. Soil Conservation Service in their publication “Soil Survey of Islands of Kauai, Oahu, Maui, Molokai and Lanai, State of Hawaii” (1972). The PkB soil type is described as dark reddish brown, sticky, plastic clay that formed in old alluvium derived from basic igneous rock. PkB is also described as having a moderate shrink-swell potential and a high shear strength. Mass grading work and development along Kailua Road have brought the project site to its present form.

### **2.2 SITE DESCRIPTION**

The project site is located along Kailua Road at the intersections with Ulua Street and Ulumanu Drive in Kailua on the Island of Oahu, Hawaii. In general, Kailua Road at these intersections is an existing four-lane divided roadway with two lanes in each direction along with several turning lanes, center median, and pedestrian walkways.

Based on our field observations, the project site appears to generally slope down from the southwest to the northeast. A topographic survey plan was not provided at the time this report was prepared. Based on Google Earth imagery, we anticipate existing ground surface elevations at the project site to range from about +104 to +90 feet Mean Sea Level (MSL) at the southwestern and northeastern portions of the project site, respectively.

At the time of our field exploration, portions of the site were generally covered by asphaltic concrete pavements, concrete walkways, and mown lawn grass. Exposed surface soils at the project site generally consisted of brown silty clay with varying amounts of sand and gravel.

### **2.3 FIELD EXPLORATION**

We explored the subsurface conditions at the project site by drilling and sampling four borings, designated as Boring Nos. 1 through 4, extending to depths ranging from about 8.5 to 21.5 feet below the existing ground surface. The borings were drilled utilizing a truck-mounted drill rig equipped with continuous flight augers. The approximate boring locations are shown on the Site Plans, Plates 2.1 and 2.2.



Our engineer monitored the drilling operations on a near continuous (full-time) basis and classified the materials encountered in the borings by visual and textural examination in the field in general accordance with ASTM D2488. These classifications were further reviewed visually and by testing in the laboratory. Soils were classified in general accordance with ASTM D2487 and the Unified Soil Classification System.

Soil samples were obtained in general accordance with ASTM D1586 by driving a 2-inch OD standard penetration sampler with a 140-pound hammer falling 30 inches. In addition, relatively undisturbed soil samples were obtained in general accordance with ASTM D3550 by driving a 3-inch OD Modified California sampler using the same hammer and drop. The blow counts needed to drive the sampler the second and third 6 inches of an 18-inch drive are shown as the "Sampling Resistance" on the Logs of Borings at the appropriate sample depths. The blow counts may need to be factored to obtain the Standard Penetration Test (SPT) blow counts.

Pocket penetrometer tests were performed on selected cohesive soil samples retrieved in the field. The pocket penetrometer test provides an indication of the unconfined compressive strength of the sample. Pocket penetrometer test results are summarized on the Logs of Borings at the appropriate sample depths.

### **2.4 SUBSURFACE CONDITIONS**

Our borings generally encountered pavement structures consisting of about 8 inches of asphaltic concrete and 4 inches of base material overlying surface fill materials, alluvial soils, and hard basalt rock formation extending down to the maximum depth explored of about 21.5 feet below the existing ground surface. The surface fill materials were encountered to depths ranging from about 2 to 5 feet below the existing ground surface and generally consisted of stiff to very stiff silty clay and medium dense silty sand with some gravel.

Alluvial soils were encountered underlying the surface fill materials to depths ranging from about 12 feet to the maximum depth explored of about 21.5 feet below the existing ground surface in Boring No. 1 and generally consisted of stiff to hard silty clay with some sand and

gravel. In addition, boulders were encountered within the alluvial soil deposits in Boring No. 2 at depths of about 2 and 6 feet below the existing ground surface.

Hard basalt rock formation was encountered underlying the alluvial soils in Boring Nos. 3 and 4 only, and extended down to the maximum depth explored in these boring of about 15.3 feet. We did not encounter groundwater in the borings at the time of our field exploration. However, it should be noted that groundwater levels are subject to change due to rainfall, time of year, seasonal precipitation, surface water runoff, and other factors. Graphic representations of the materials encountered are presented on the Logs of Borings, Appendix A.

## **2.5 LABORATORY TESTING**

Moisture Content (ASTM D2216) and Unit Weight (ASTM D2937) determinations were performed on selected samples as an aid in the classification and evaluation of soil properties. The test results are presented on the Logs of Borings at the appropriate sample depths.

Two Atterberg Limits tests (ASTM D4318) were performed on selected soil samples to evaluate the liquid and plastic limits. The samples tested generally had high Plasticity Indexes (PIs) of about 28 and 32 and plotted as high plasticity clay (CH) on a Standard Plasticity Chart. The test results are summarized on the Logs of Borings at the appropriate sample depths. Graphic presentations of the Atterberg Limits test results are provided on Plate B-1.

Two one-inch Ring Swell tests were performed on relatively undisturbed (natural) and remolded samples to evaluate the swelling potential of the on-site soils. Swell test results of about 2.1 and 8.9 percent were observed for the relatively undisturbed (natural) and remolded samples, respectively, indicating the on-site soils have a moderately high to high swelling potential when subjected to moisture fluctuations. The Ring Swell test results are summarized on Plate B-2.

One Unconfined Compression test (ASTM D2166) was performed on a selected in-situ cohesive soil sample to evaluate the unconfined compressive strength of the soil. The test

## SECTION 2.0 SITE CHARACTERIZATION AND FINDINGS

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resulted in an unconfined compressive strength of about 1.8 kips per square foot (ksf). The Unconfined Compression test results are presented on Plate B-3.

### 2.6 SEISMIC DESIGN CONSIDERATIONS

Based on the International Building Code, 2018 Edition (IBC 2018) and American Society of Civil Engineers Standard ASCE/SEI 7-16 (ASCE 7-16), the project site may be subject to seismic activity, and seismic design considerations will need to be addressed. Based on the subsurface materials encountered at the project site and the geologic setting of the area, we anticipate the project site may be classified from a seismic analysis standpoint as being a “Stiff Soil Profile” site corresponding to a Site Class D soil profile type based on Chapter 20 of ASCE 7-16.

Based on Site Class D, the following seismic design parameters were estimated and may be used for seismic analysis of the project.

SUMMARY OF SEISMIC DESIGN PARAMETERS	
Mapped MCE Spectral Response Acceleration, $S_s$	0.552g
Mapped MCE Spectral Response Acceleration, $S_1$	0.155g
Site Class	D
Site Coefficient, $F_a$	1.358
Site Coefficient, $F_v$	2.291
Design Spectral Response Acceleration, $S_{D5}$	0.500g
Design Spectral Response Acceleration, $S_{D1}$	0.236g
Peak Ground Acceleration, PGA	0.254g
Site Modified Peak Ground Acceleration, $PGA_M$	0.342g

Based on the subsurface conditions encountered, the phenomenon of soil liquefaction is not a design consideration for this project site.

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*END OF SITE CHARACTERIZATION AND FINDINGS*

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### SECTION 3.0 DISCUSSION AND RECOMMENDATIONS

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Based on the results of our field exploration, the project site is generally underlain by pavement structures consisting of about 8 inches of asphaltic concrete and 4 inches of base material overlying surface fill materials, alluvial soils, and hard basalt rock formation extending down to the maximum depth explored of about 21.5 feet below the existing ground surface. The surface fill materials were encountered to depths ranging from about 2 to 5 feet below the existing ground surface and generally consisted of stiff to very stiff silty clay and medium dense silty sand with some gravel.

Alluvial soils were encountered underlying the surface fill materials to depths ranging from about 12 feet to the maximum depth explored of about 21.5 feet below the existing ground surface in Boring No. 1 and generally consisted of stiff to hard silty clay with some sand and gravel. In addition, boulders were encountered within the alluvial soil deposits in Boring No. 2 at depths of about 2 and 6 feet below the existing ground surface.

Hard basalt rock formation was encountered underlying the alluvial soils in Boring Nos. 3 and 4 only, and extended down to the maximum depth explored in these boring of about 15.3 feet. We did not encounter groundwater in the borings at the time of our field exploration. However, it should be noted that groundwater levels are subject to change due to rainfall, time of year, seasonal precipitation, surface water runoff, and other factors. In addition, subterranean seepage may be encountered during construction due to high rainfall in the area, sloping terrain and relict structure in the alluvial soils and basalt rock formation encountered.

Based on the loading demands provided by the project structural engineer and the subsurface soil conditions encountered at the site, we believe the new traffic signal structures may be supported by a deep foundation system consisting of cast-in-place concrete drilled shafts. Detailed discussion of these items and our geotechnical recommendations for design of drilled shaft foundations and other geotechnical aspects of the project are further discussed in the following sections.

### **3.1 DRILLED SHAFT FOUNDATIONS**

In order to develop the required bearing and lateral load resistances, we believe the new traffic signal structures may be supported by a deep foundation system consisting of cast-in-place concrete drilled shafts. In general, drilled shaft foundations are constructed by drilling a hole down into the bearing strata, placing reinforcing steel, and then pumping high slump concrete to fill up the hole. The result is a cast-in-place concrete drilled shaft for foundation support.

Based on the bolt circle diameter and the square bearing plate dimensions, we envision that drilled shaft foundations with minimum diameters of 24 and 42 inches may be used to support the new Type 1 and Type 2 traffic signal structures, respectively. Detailed discussions and recommendations for foundation design are presented in the following subsections.

#### **3.1.1 LATERAL LOAD RESISTANCE**

The lateral load resistance of drilled shafts is a function of the stiffness of the surrounding soil, the stiffness of the drilled shaft, allowable deflection at the top of the drilled shaft, and the induced moment in the drilled shaft. The lateral load analyses were performed using the program LPILE, which is a microcomputer adaptation of a finite difference, laterally loaded deep foundation program originally developed at the University of Texas at Austin.

The program solves for deflection and bending moment along a deep foundation under lateral loads as a function of depth. The analysis was carried out with the use of non-linear “p-y” curves to represent soil moduli. The lateral deflection was then computed using the appropriate soil moduli at various depths.

Based on the provided preliminary structural loads and the anticipated subsurface soil conditions at each site, we performed the lateral load analyses for the above drilled shaft foundations. The results of our analyses are summarized in the tables below.

In general, we recommend installing 24-inch diameter drilled shaft foundations with minimum embedment lengths of 6 feet below the design finished grade to support the

new Type 1 traffic signal structures planned for the project. In addition, we recommend installing 42-inch diameter drilled shaft foundations with minimum embedment lengths of 12 feet below the design finished grade to support the new Type 2 traffic signal structures planned for the project. The project structural engineer should verify the drilled shaft structural capacity for the calculated induced stresses.

<b>FOUNDATION ANALYSES FOR TYPE 1 TRAFFIC SIGNAL POLE STRUCTURE</b>				
<b>Minimum Drilled Shaft Diameter (inches)</b>	<b>Minimum Drilled Shaft Length (feet)</b>	<b>Lateral Deflection (inches)</b>	<b>Maximum Induced Moment (kip-foot)</b>	<b>Depth to Maximum Moment (feet)</b>
24	6	0.1	6.4	1.6

<b>FOUNDATION ANALYSES FOR TYPE 2 TRAFFIC SIGNAL POLE STRUCTURE WITH 30-FOOT MAST ARM</b>				
<b>Minimum Drilled Shaft Diameter (inches)</b>	<b>Minimum Drilled Shaft Length (feet)</b>	<b>Lateral Deflection (inches)</b>	<b>Maximum Induced Moment (kip-foot)</b>	<b>Depth to Maximum Moment (feet)</b>
42	12	0.3	108.3	3.5

3.1.2 FOUNDATION SETTLEMENT

Settlement of the drilled shaft foundations will result primarily from elastic compression of the shaft member and subgrade response. We estimate the total settlement of the drilled shaft supported foundations to be 0.5 inches or less with differential settlements between drilled shafts not exceeding about one half of the total settlement. We believe these settlements are essentially elastic and should occur as the loads are applied.

3.1.3 DRILLED SHAFT CONSTRUCTION CONSIDERATIONS

In general, the performance of drilled shafts depends significantly upon the contractor's method of installation and construction procedures. The load bearing capacities of the drilled shafts depend, to a significant extent, on the friction between the shaft and the surrounding soils. Therefore, proper construction techniques, especially during the drilling operations, are important. The contractor should exercise care in drilling the shaft holes and placing concrete into the drilled holes.

It should be noted that cobbles, boulders, and hard basalt rock formation may be encountered within the depth of the drilled shafts. Therefore, some difficult drilling conditions may be expected. The drilled shaft subcontractor should have the appropriate equipment and tools to drill through these types of natural obstructions, where encountered. The drilled shaft subcontractor should demonstrate that the proposed drilling equipment will be capable of installing the drilled shafts to the recommended depths and dimensions.

Based on the estimated lengths of the drilled shafts, groundwater is generally not expected in the drilled hole during the shaft installation. However, concrete placement by tremie method is recommended during construction of the drilled shafts in lieu of free fall method. This is to reduce the potential for concrete from striking the steel reinforcement cage or shaft sidewalls during concrete placement.

A low-shrink concrete mix with high slump (7 to 9-inch slump range) should be used to provide close contact between the drilled shaft and the surrounding soils. Due to factors such as seasonal rainfall and perched water, groundwater may be encountered in the drilled hole. The concrete should be placed in a suitable manner to reduce the potential for segregation of the aggregates from the concrete mix. In addition, the concrete should be placed promptly after drilling (within 24 hours after drilling of the holes) to reduce the potential for softening of the sides of the drilled hole.

We recommend a specialty contractor experienced in the construction of drilled shaft foundations (minimum five projects) perform the installation of the drilled shafts. Due to the specialized nature of the drilled shaft foundation construction, observation and testing of the drilled shaft foundation system should be designated as a “Special Inspection” item. Therefore, a Kokua Geotech LLC representative (Special Inspector) should be present to observe the geotechnical aspects of the drilled shaft foundation construction.

### **3.2 UTILITY TRENCHES**

We anticipate that new underground utility lines may be required for the project. As discussed above, all excavations should be made in accordance with applicable Occupational Safety and Health Administration (OSHA) and state regulations. The contractor should determine the method and equipment to be used for utility trench excavation, subject to practical limits and safety considerations. In addition, the trench excavations should comply with the applicable federal, state, and local safety requirements. The contractor should be responsible for trench shoring design and installation.

Based on our borings, trench excavations will likely encounter surficial fill materials and alluvial soils generally consisting of stiff to hard silty clay and medium dense silty sand with some gravel. In addition, cobbles, boulders, and basalt rock formation may be encountered within the depth of the excavations. It is anticipated that most of the material may be excavated with normal heavy excavation equipment. However, deep excavations and excavations encountering boulders may require the use of hoerams.

In general, we recommend providing granular bedding consisting of 6 inches of open-graded gravel, such as No. 3 Fine gravel (ASTM C33, No. 67 gradation), under the pipes for uniform support. In addition, open-graded gravel (ASTM C33, No. 67 gradation) should also be used for the initial trench backfill up to about 12 inches above the pipes to provide adequate support around the pipes. It is critical to use a free-draining material, such as open-graded gravel, to reduce the potential for formation of voids below the haunches of pipes and to provide



adequate support for the sides of the pipes. Improper trench backfill could result in backfill settlement and pipe damage.

Trench subgrades should firm and unyielding prior to placing the minimum 6-inch thick layer of open-graded gravel below the pipes. Soft and/or loose materials encountered at the bottom of trench excavations should be over-excavated to expose the underlying firm materials. The over-excavation should be backfilled with general fill materials compacted to a minimum of 90 percent relative compaction. Before the placement of bedding material, a Kokua Geotech LLC representative should observe the excavated trench bottom to confirm that firm materials are exposed at the bottom of the trench.

Trench backfill material above the open-graded gravel may consist of general fill materials (on-site soils with rock fragments less than 3 inches in largest dimension). The backfill should be placed in maximum 8-inch level loose lifts and mechanically compacted to no less than 90 percent relative compaction to reduce the potential for appreciable future ground subsidence. The upper 2 feet below the finished grade in areas subjected to vehicular traffic should be compacted to a minimum of 95 percent relative compaction.

Relative compaction refers to the in-place dry density of soil expressed as a percentage of the maximum dry density of the same soil determined in accordance with ASTM D1557. Optimum moisture is the water content (percentage by dry weight) corresponding to the maximum dry density.

### **3.3 SITE DRAINAGE CONSIDERATIONS**

The drainage condition around the new traffic signal structures is critical to maintaining proper foundation performance because ponded water could cause subsurface soil saturation and subsequent heaving or loss of strength. Finished grades outside the new structures should be sloped to shed water away from the slabs and foundations and to reduce the potential for ponding around the structure.

## SECTION 3.0 DISCUSSION AND RECOMMENDATIONS

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Drainage systems and finished grades for the project site should be designed by a Licensed Civil Engineer so that surface runoff is directed away from the structures. Drainage swales should be provided as soon as possible and should be maintained to drain surface water runoff away from the foundations.

### **3.4 DESIGN REVIEW AND CONSTRUCTION OBSERVATION SERVICES**

The construction plans and specifications for the project should be forwarded to us for review to determine whether the recommendations contained in this report are adequately reflected in those documents. If this review is not made, Kokua Geotech LLC cannot assume responsibility for misinterpretation of our recommendations.

Kokua Geotech LLC should also be retained to monitor the drilled shaft foundation installation operations and other aspects of earthwork construction to determine whether the recommendations of this report are followed. The recommendations presented herein are contingent upon such observations. If the actual exposed subsurface soil conditions encountered during construction differ from those assumed or considered in this report, Kokua Geotech LLC should be contacted to review and/or revise the geotechnical recommendations presented herein.

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*END OF DISCUSSION AND RECOMMENDATIONS*

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## SECTION 4.0 LIMITATIONS

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This report has been prepared for the exclusive use of Community Planning and Engineering, Inc. and their project consultants for specific application to the design of the *Kailua Road Intersection Improvements, Vicinity of Uluoa Street and Ulumanu Drive* project in accordance with generally accepted geotechnical engineering principles and practices. No warranty is expressed or implied. If any part of the project concept is altered or if subsurface conditions differ from those described in this report, then the information presented herein shall be considered invalid, unless the changes are reviewed, and any supplemental or revised recommendations issued in writing by Kokua Geotech LLC.

The analyses and report recommendations are based in part upon information obtained from the field borings and the assumption that subsurface conditions do not vary significantly from those observed in the borings. Variations of the subsurface conditions between and beyond the field borings may occur, and the nature and extent of these variations may not become evident until construction is underway. If variations then appear evident, Kokua Geotech LLC should be notified so that we can re-evaluate the recommendations presented herein.

The owner/client should be aware that unanticipated soil conditions are commonly encountered. Unforeseen subsurface conditions, such as perched groundwater, soft deposits, hard layers or cavities, may occur in localized areas and may require additional probing or corrections in the field (which may result in construction delays) to attain a properly constructed project. Therefore, a sufficient contingency fund is recommended to accommodate these possible extra costs.

The field boring locations indicated herein are approximate, having been estimated by taping from visible features shown on the Traffic Signal Plan transmitted by Community Planning and Engineering, Inc. on October 6, 2023. A topographic survey plan was not provided at the time this report was prepared. Elevations of the borings were estimated from Google Earth imagery.

## SECTION 4.0 LIMITATIONS

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The field boring locations and elevations should be considered accurate only to the degree implied by the methods used.

The stratification breaks shown on the graphic representations of the borings depict the approximate boundaries between soil types and, as such, may denote a gradual transition. We did not encounter groundwater in the borings at the time of our field exploration. However, it should be noted that groundwater levels are subject to change due to rainfall, time of year, seasonal precipitation, surface water runoff, and other factors. These data have been reviewed and interpretations made in the formulation of this report.

This report has been prepared solely for the purpose of assisting the design engineers in the design of the project. Therefore, this report may not contain sufficient data, or the proper information, to serve as a basis for detailed construction cost estimates.

This geotechnical engineering exploration conducted at the project site was not intended to investigate the potential presence of hazardous materials existing at the project site. It should be noted that the equipment, techniques, and personnel used to conduct a geo-environmental exploration differ substantially from those applied in geotechnical engineering.

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*END OF LIMITATIONS*

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**CLOSURE**

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The following plates and appendices are attached and complete this report:

Project Location Map ..... Plate 1  
Site Plans ..... Plates 2.1 through 2.4  
Logs of Borings ..... Appendix A  
Laboratory Test Results ..... Appendix B

This report concludes our scope of work outlined in our fee proposal dated November 10, 2022. If you have any questions regarding this report or if any part of the report is not clear, please contact our office.

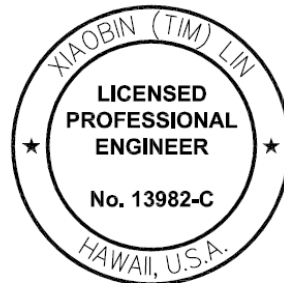
Respectfully submitted,

**Kokua Geotech LLC**



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**Xiaobin (Tim) Lin, P.E.**  
President



THIS WORK WAS PREPARED BY  
ME OR UNDER MY SUPERVISION.  
(MY LICENSE EXPIRES 4/30/2024)

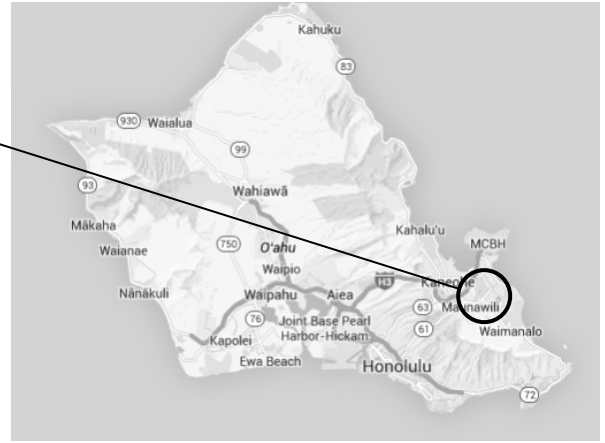
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## PLATES

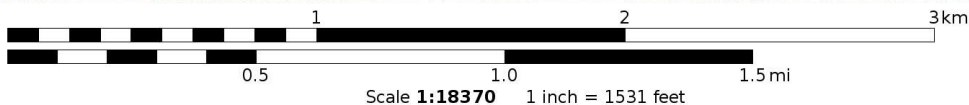
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GENERAL PROJECT LOCATION



Mercator Projection  
WGS84  
UTM Zone 4Q  
 CALTOPO



**PROJECT LOCATION MAP**

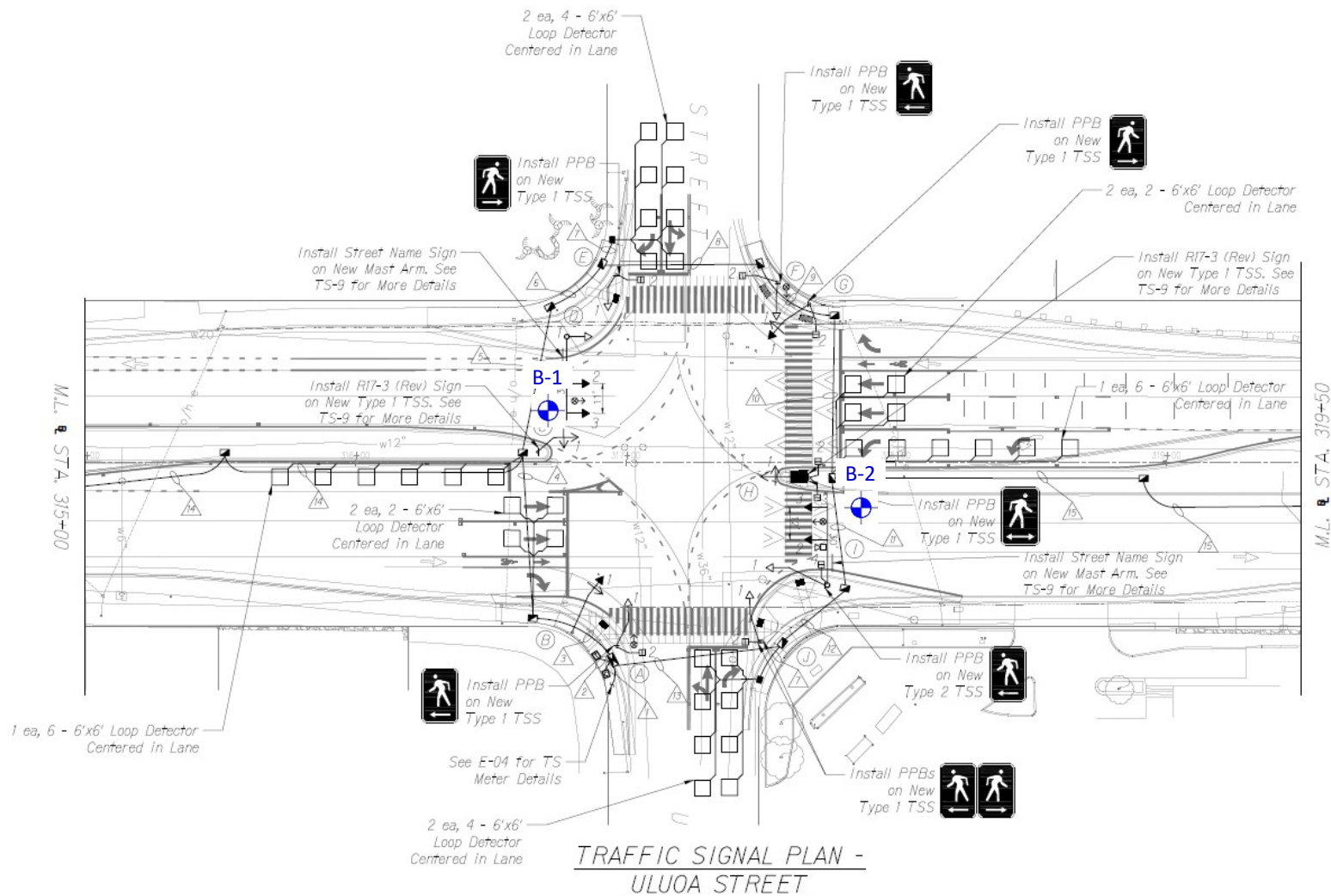
KAILUA ROAD INTERSECTION IMPROVEMENTS  
VICINITY OF ULUOA STREET AND ULUMANU DRIVE  
KAILUA, OAHU, HAWAII

PROJECT NO.: 110922-00

DATE: DECEMBER 2023

PLATE

**1**

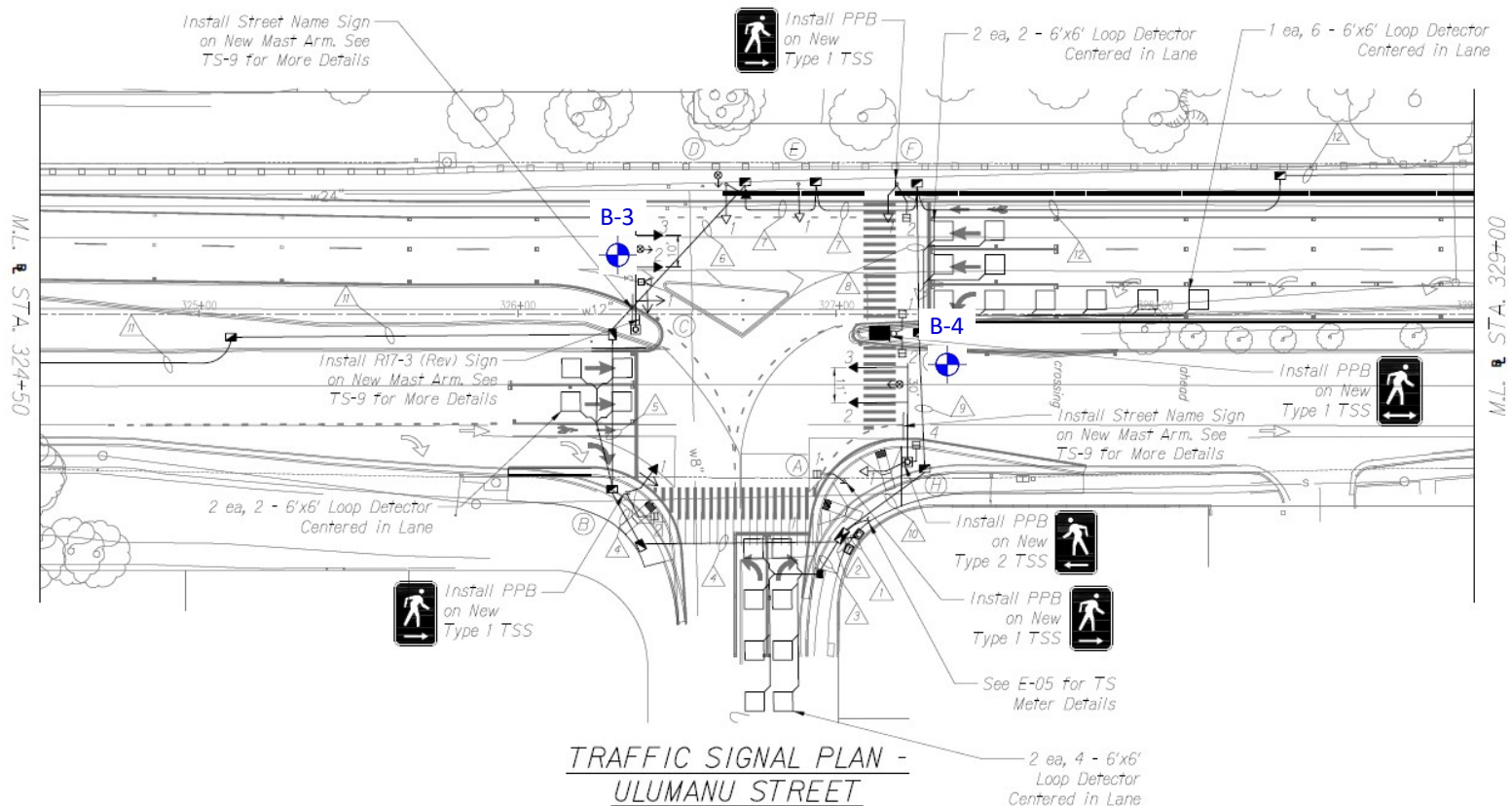
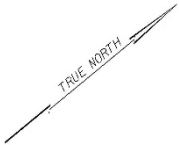


REFERENCE: TRAFFIC SIGNAL PLAN – ULUOA STREET TRANSMITTED BY COMMUNITY PLANNING AND ENGINEERING INC. ON OCTOBER 6, 2023

 APPROXIMATE BORING LOCATION

<b>SITE PLAN – 1</b>	
KAILUA ROAD INTERSECTION IMPROVEMENTS VICINITY OF ULUOA STREET AND ULUMANU DRIVE KAILUA, OAHU, HAWAII	
PROJECT NO.: 110922-00	<b>PLATE 2.1</b>
DATE: DECEMBER 2023	





REFERENCE: TRAFFIC SIGNAL PLAN – ULUOA STREET TRANSMITTED BY COMMUNITY PLANNING AND ENGINEERING INC. ON OCTOBER 6, 2023

 APPROXIMATE BORING LOCATION

<b>SITE PLAN – 2</b>	
KAILUA ROAD INTERSECTION IMPROVEMENTS VICINITY OF ULUOA STREET AND ULUMANU DRIVE KAILUA, OAHU, HAWAII	
PROJECT NO.: 110922-00	<b>PLATE 2.2</b>
DATE: DECEMBER 2023	

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## APPENDIX A

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Project: **Kailua Road Intersection Improvements**  
 Project Location: **Kailua, Oahu, Hawaii**  
 Project Number: **110922-00**

**Kokua Geotech LLC**  
 94-974 Pakela Street, Suite 109  
 Waipahu, HI 96797  
 (808) 397-6974

**Key to Log of Borings**  
 Sheet 1 of 1

Elevation (feet)	Depth (feet)	Sample Type	Sample Number	Sampling Resistance, blows/ft	U.S.C.S	Graphic Log	MATERIAL DESCRIPTION	Pocket Pen./Torvane, tsf	Water Content, %	Dry Unit Weight, pcf	Remarks and Other Tests
1	2	3	4	5	6	7	8	9	10	11	12

**COLUMN DESCRIPTIONS**

- 1** Elevation (feet): Elevation (MSL, feet).
- 2** Depth (feet): Depth in feet below the ground surface.
- 3** Sample Type: Type of soil sample collected at the depth interval shown.
- 4** Sample Number: Sample identification number.
- 5** Sampling Resistance, blows/ft: Number of blows to advance driven sampler one foot (or distance shown) beyond seating interval using the hammer identified on the boring log.
- 6** U.S.C.S: Type of material encountered.
- 7** Graphic Log: Graphic depiction of the subsurface material encountered.
- 8** MATERIAL DESCRIPTION: Description of material encountered. May include consistency, moisture, color, and other descriptive text.
- 9** Pocket Pen./Torvane, tsf: the reading from Poocket Penetrometer or Torvane.
- 10** Water Content, %: Water content of the soil sample, expressed as percentage of dry weight of sample.
- 11** Dry Unit Weight, pcf: Dry weight per unit volume of soil sample measured in laboratory, in pounds per cubic foot.
- 12** Remarks and Other Tests: Other Tests

**FIELD AND LABORATORY TEST ABBREVIATIONS**

- CHEM: Chemical tests to assess corrosivity
- COMP: Compaction test
- CONS: One-dimensional consolidation test
- LL: Liquid Limit, percent
- PI: Plasticity Index, percent
- SA: Sieve analysis (percent passing No. 200 Sieve)
- UC: Unconfined compressive strength test, Qu, in ksf
- WA: Wash sieve (percent passing No. 200 Sieve)

**MATERIAL GRAPHIC SYMBOLS**

- Asphaltic Concrete (AC)
- Basalt Rock Formation
- Boulders
- Fat CLAY, CLAY w/SAND, SANDY CLAY (CH)
- Silty GRAVEL (GM)
- Silty SAND (SM)

**TYPICAL SAMPLER GRAPHIC SYMBOLS**

- Auger sampler
- Bulk Sample
- 3-inch-OD California w/ brass rings
- CME Sampler
- Grab Sample
- HQ Coring
- 3-inch OD Modified California w/ brass liners
- Pitcher Sample
- PQ Coring
- Probing w/Pointed Tip
- 2-inch-OD unlined split spoon (SPT)
- Shelby Tube (Thin-walled, fixed head)

**OTHER GRAPHIC SYMBOLS**

- Water level (at time of drilling, ATD)
- Water level (after waiting)
- Minor change in material properties within a stratum
- Inferred/gradational contact between strata
- Queried contact between strata

**GENERAL NOTES**

- 1: Soil classifications are based on the Unified Soil Classification System. Descriptions and stratum lines are interpretive, and actual lithologic changes may be gradual. Field descriptions may have been modified to reflect results of lab tests.
- 2: Descriptions on these logs apply only at the specific boring locations and at the time the borings were advanced. They are not warranted to be representative of subsurface conditions at other locations or times.

Project: <b>Kailua Road Intersection Improvements</b>	<b>Kokua Geotech LLC</b> 94-974 Pakela Street, Suite 109 Waipahu, HI 96797 (808) 397-6974	<b>Log of Boring No. 1</b> Sheet 1 of 1
Project Location: <b>Kailua, Oahu, Hawaii</b>		
Project Number: <b>110922-00</b>		

Date(s) Drilled: <b>11/27/23</b>	Logged By: <b>CH</b>	Checked By: <b>AJF</b>
Drilling Method: <b>CF Auger</b>	Drill Bit Size/Type: <b>4-inch Solid Stem Auger</b>	Total Depth of Borehole: <b>21.5 feet</b>
Drill Rig Type: <b>Mobile B-53</b>	Drilling Contractor: <b>Kokua Geotech LLC</b>	Approximate Surface Elevation: <b>+103 feet MSL*</b>
Groundwater Level and Date Measured: <b>Not Encountered</b>	Sampling Method(s): <b>MCS &amp; SPT</b>	Hammer Data: <b>140 lbs. with 30-inch drop</b>
Borehole Backfill: <b>Soil Cuttings, Gravel, and AC Patch</b>	Location: <b>See Site Plan (Plate 2.1)</b>	

Elevation (feet)	Depth (feet)	Sample Type	Sample Number	Sampling Resistance, blows/ft	U.S.C.S	Graphic Log	MATERIAL DESCRIPTION	Pocket Pen./Torvane, tsf	Water Content, %	Dry Unit Weight, pcf	Remarks and Other Tests
103	0						8-inch ASPHALTIC CONCRETE				
			1	25	GM CH		Grayish brown SILTY GRAVEL with some sand, medium dense, moist (base material)	3.0	32	90	Sw.= 2.1%
			2	10			Reddish brown SILTY CLAY with some sand and gravel (coralline), stiff, moist (fill)		31		
98	5		3	19	CH		Reddish brown to brown SILTY CLAY with some sand and a little gravel, stiff, moist (alluvium)	2.5	38	78	UC= 1.8ksf
93	10		4	11			grades to very stiff		39		LL=59, PI=32
88	15		5	29	CH		Reddish brown with multi-color mottling SILTY CLAY with some sand and decomposed gravel, very stiff, moist (alluvium)	3.5	44	80	
83	20		6	10			grades to stiff		51		
							Boring terminated at approximately 21.5 feet below the existing ground surface				
							*Elevations of borings estimated from Google Earth imagery				
78	25										

Project: <b>Kailua Road Intersection Improvements</b>	<b>Kokua Geotech LLC</b> 94-974 Pakela Street, Suite 109 Waipahu, HI 96797 (808) 397-6974	<b>Log of Boring No. 2</b> Sheet 1 of 1
Project Location: <b>Kailua, Oahu, Hawaii</b>		
Project Number: <b>110922-00</b>		

Date(s) Drilled: <b>11/27/23</b>	Logged By: <b>CH</b>	Checked By: <b>AJF</b>
Drilling Method: <b>CF Auger</b>	Drill Bit Size/Type: <b>4-inch Solid Stem Auger</b>	Total Depth of Borehole: <b>8.5 feet</b>
Drill Rig Type: <b>Mobile B-53</b>	Drilling Contractor: <b>Kokua Geotech LLC</b>	Approximate Surface Elevation: <b>+102 feet MSL*</b>
Groundwater Level and Date Measured: <b>Not Encountered</b>	Sampling Method(s): <b>MCS &amp; SPT</b>	Hammer Data: <b>140 lbs. with 30-inch drop</b>
Borehole Backfill: <b>Soil Cuttings, Gravel, and AC Patch</b>	Location: <b>See Site Plan (Plate 2.1)</b>	

Elevation (feet)	Depth (feet)	Sample Type	Sample Number	Sampling Resistance, blows/ft	U.S.C.S	Graphic Log	MATERIAL DESCRIPTION	Pocket Pen./Torvane, tsf	Water Content, %	Dry Unit Weight, pcf	Remarks and Other Tests
102	0						8-inch ASPHALTIC CONCRETE				
			1	30/9" +10/0" Ref.	GM CH		Grayish brown SILTY GRAVEL with some sand, medium dense, moist (base material)	2.0	34	100	
			2	20/0" Ref.			Light brown SILTY CLAY with some sand and gravel, stiff, moist (fill)		-		
					CH		Gray BOULDER, hard (alluvium)				
							Brown SILTY CLAY with some sand and gravel, hard, moist (alluvium)				
97	5		3	72			Gray BOULDER, hard (alluvium)		31		
			4	20/0" Ref.			Gray BOULDER, hard (alluvium)		-		
							Boring terminated at approximately 7.0 feet below the existing ground surface on an apparent hard boulder				
92	10										
87	15										
82	20										
77	25										

Project: <b>Kailua Road Intersection Improvements</b>	<b>Kokua Geotech LLC</b> 94-974 Pakela Street, Suite 109 Waipahu, HI 96797 (808) 397-6974	<b>Log of Boring No. 3</b> Sheet 1 of 1
Project Location: <b>Kailua, Oahu, Hawaii</b>		
Project Number: <b>110922-00</b>		

Date(s) Drilled: <b>11/28/23</b>	Logged By: <b>CH</b>	Checked By: <b>AJF</b>
Drilling Method: <b>CF Auger</b>	Drill Bit Size/Type: <b>4-inch Solid Stem Auger</b>	Total Depth of Borehole: <b>13.5 feet</b>
Drill Rig Type: <b>Mobile B-53</b>	Drilling Contractor: <b>Kokua Geotech LLC</b>	Approximate Surface Elevation: <b>+88 feet MSL*</b>
Groundwater Level and Date Measured: <b>Not Encountered</b>	Sampling Method(s): <b>MCS &amp; SPT</b>	Hammer Data: <b>140 lbs. with 30-inch drop</b>
Borehole Backfill: <b>Soil Cuttings, Gravel, and AC Patch</b>	Location: <b>See Site Plan (Plate 2.2)</b>	

Elevation (feet)	Depth (feet)	Sample Type	Sample Number	Sampling Resistance, blows/ft	U.S.C.S.	Graphic Log	MATERIAL DESCRIPTION	Pocket Pen./Torvane, tsf	Water Content, %	Dry Unit Weight, pcf	Remarks and Other Tests
88	0						8-inch ASPHALTIC CONCRETE				
			1	34	GM SM CH		Grayish brown SILTY GRAVEL with some sand, medium dense, moist (base material)		8	131	
			2	25			Brownish tan SILTY SAND with some gravel, medium dense, moist (fill)		14		
			3	9	CH		Brownish tan SILTY CLAY with some sand, gravel, and cobbles, very stiff, moist (fill)				
83	5		3				Brown SILTY CLAY with some sand and a little gravel, stiff, moist (alluvium)	1.5	26	100	Sw.= 8.9%
			4	31					21		
			5	10/0" Ref.			Grayish brown BASALT, moderately weathered, hard (basalt rock formation)				
							Boring terminated at approximately 13.5 feet below the existing ground surface on apparent hard basalt rock formation				
73	15										
68	20										
63	25										

Project: <b>Kailua Road Intersection Improvements</b>	<b>Kokua Geotech LLC</b> 94-974 Pakela Street, Suite 109 Waipahu, HI 96797 (808) 397-6974	<b>Log of Boring No. 4</b> Sheet 1 of 1
Project Location: <b>Kailua, Oahu, Hawaii</b>		
Project Number: <b>110922-00</b>		

Date(s) Drilled: <b>11/28/23</b>	Logged By: <b>CH</b>	Checked By: <b>AJF</b>
Drilling Method: <b>CF Auger</b>	Drill Bit Size/Type: <b>4-inch Solid Stem Auger</b>	Total Depth of Borehole: <b>15.3 feet</b>
Drill Rig Type: <b>Mobile B-53</b>	Drilling Contractor: <b>Kokua Geotech LLC</b>	Approximate Surface Elevation: <b>+91 feet MSL*</b>
Groundwater Level and Date Measured: <b>Not Encountered</b>	Sampling Method(s): <b>MCS &amp; SPT</b>	Hammer Data: <b>140 lbs. with 30-inch drop</b>
Borehole Backfill: <b>Soil Cuttings, Gravel, and AC Patch</b>	Location: <b>See Site Plan (Plate 2.2)</b>	

Elevation (feet)	Depth (feet)	Sample Type	Sample Number	Sampling Resistance, blows/ft	U.S.C.S.	Graphic Log	MATERIAL DESCRIPTION	Pocket Pen./Torvane, tsf	Water Content, %	Dry Unit Weight, pcf	Remarks and Other Tests
91	0						8-inch ASPHALTIC CONCRETE				
			1	30	GM SM		Grayish brown SILTY GRAVEL with some sand, medium dense, moist (base material)		14	110	
			2	23	CH		Brownish gray SILTY SAND with some gravel, medium dense, moist (fill)		30		LL=57, PI=28
			3	32			Reddish brown SILTY CLAY with some sand and gravel, hard, moist (alluvium)		27		
86	5										
			4	47			Grayish brown BASALT, highly to moderately weathered, hard (basalt rock formation)		33		
76	15		5	30/3" Ref.			Boring terminated at approximately 15.3 feet below the existing ground surface on apparent hard basalt rock formation		-		
71	20										
66	25										

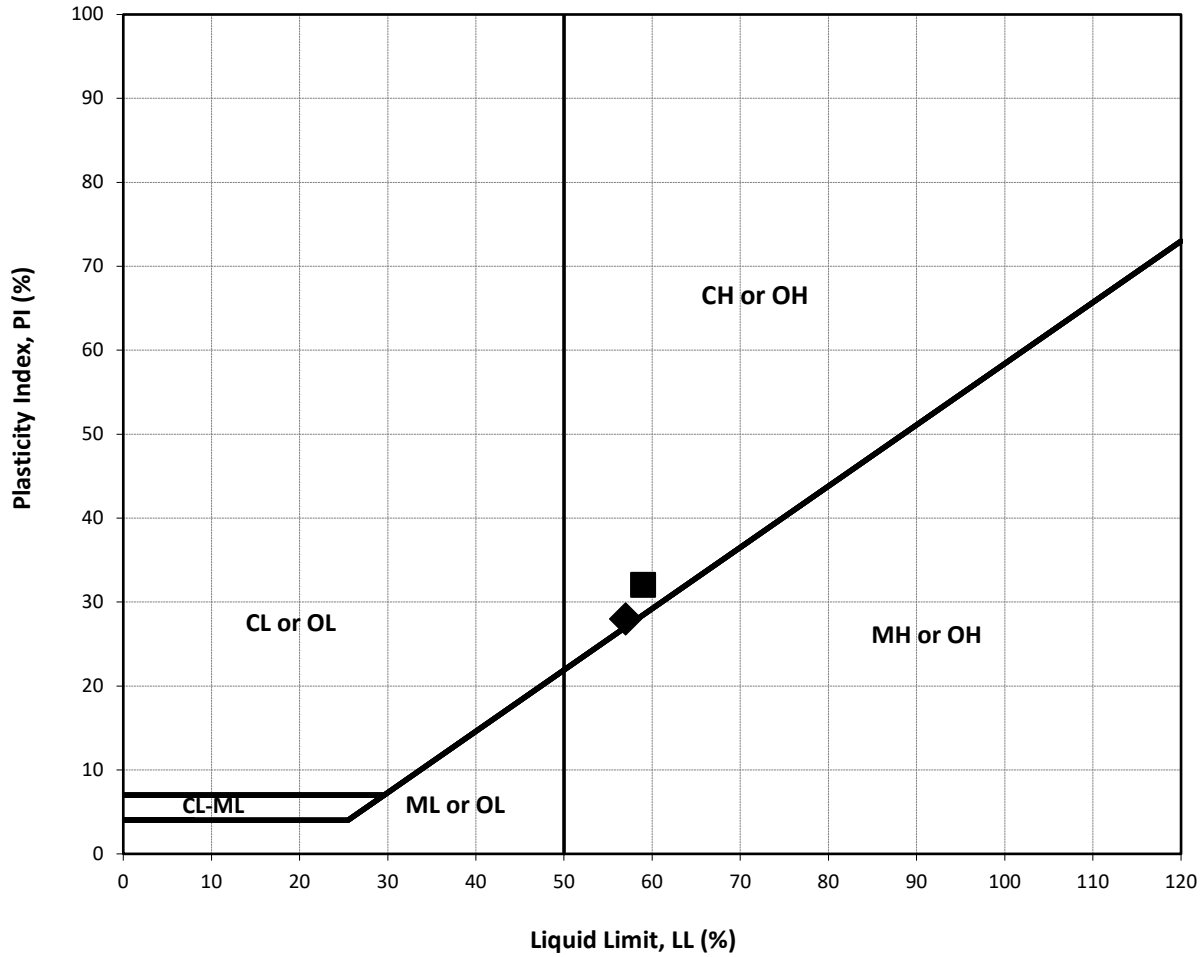
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## **APPENDIX B**

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**PLASTICITY CHART**



Symbol	Sample	Depth (feet)	Material Description	USCS	LL	PL	PI
■	B-1	10.0 to 11.5	Reddish brown SILTY CLAY with some sand and gravel	CH	59	27	32
◆	B-4	2.5 to 4.0	Reddish brown SILTY CLAY with some sand and gravel	CH	57	29	28

**SUMMARY OF ATTERBERG LIMITS (ASTM D4318) TEST RESULTS**

 <b>Kokua Geotech LLC</b> Soil and Foundation Engineering	KAILUA ROAD INTERSECTION IMPROVEMENTS VICINITY OF ULUOA STREET AND ULUMANU DRIVE KAILUA, OAHU, HAWAII	
	PROJECT NO.: 110922-00	<b>PLATE</b> <b>B-1</b>
	DATE: DECEMBER 2023	

<u>Location</u>	<u>Depth</u> (feet)	<u>Test Type</u>	<u>Soil Description</u>	<u>Dry Density</u> (pcf)	<u>Moisture Contents</u>			<u>Ring Swell</u> (%)
					<u>Initial</u> (%)	<u>Air-Dried</u> (%)	<u>Final</u> (%)	
B-1	1.0 to 2.5	Natural	Reddish brown SILTY CLAY with some sand and gravel	92.8	34.0	21.2	38.8	2.1
B-3	5.0 to 6.5	Remolded	Brown SILTY CLAY with some sand and a little gravel	100.5	22.6	12.8	28.8	8.9

Note: Samples tested were either relatively undisturbed (natural) or remolded in 2.4-inch diameter by 1-inch high rings. Samples were then air-dried overnight followed by saturating for a minimum of 24 hours under a surcharge pressure of 60 psf.

### SUMMARY OF RING SWELL TEST RESULTS

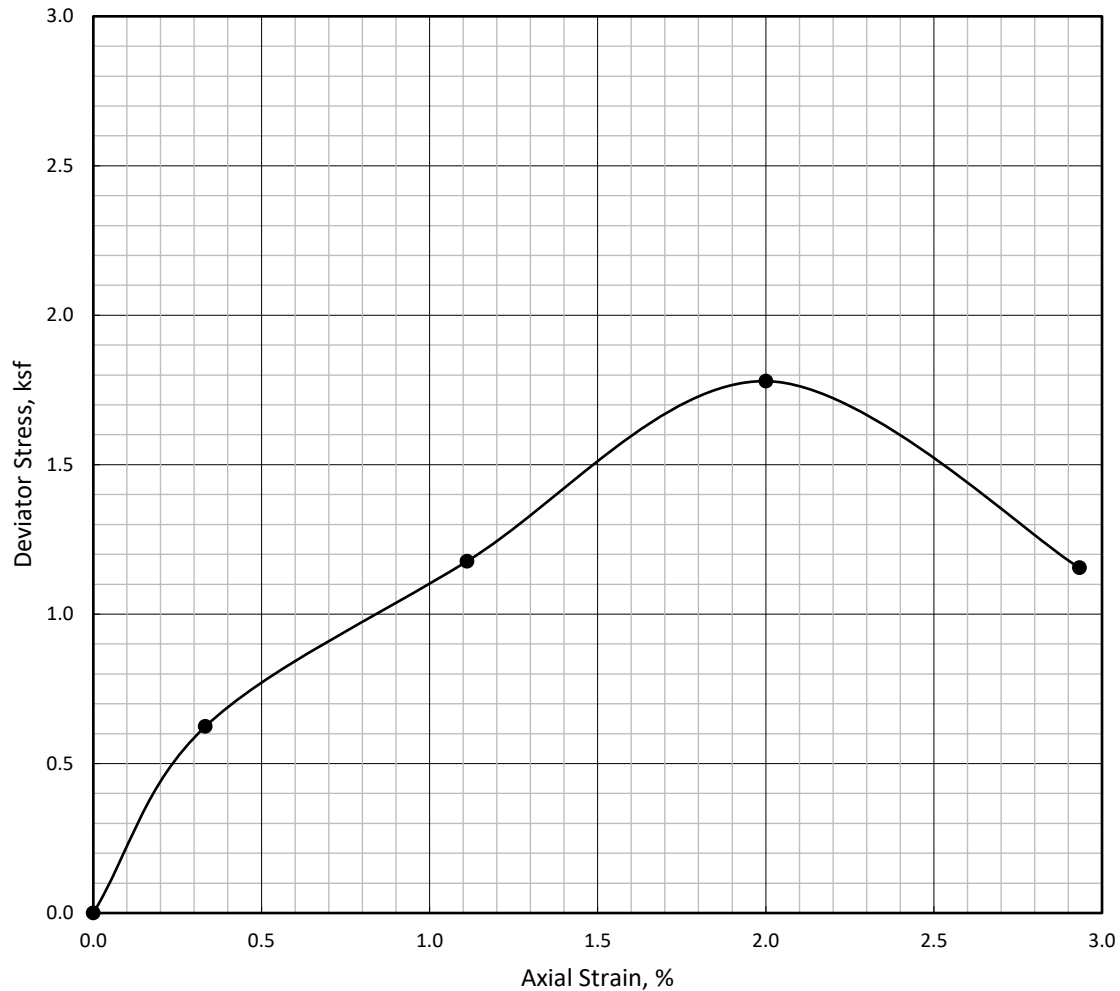
KAILUA ROAD INTERSECTION IMPROVEMENTS  
VICINITY OF ULUOA STREET AND ULUMANU DRIVE  
KAILUA, OAHU, HAWAII



PROJECT NO.: 110922-00

DATE: DECEMBER 2023

PLATE  
**B-2**



Location: B-1  
 Depth: 5.0 to 6.5 feet  
 Description: Reddish brown to brown SILTY CLAY w/sm sand & a little gravel

Dry Density: 77.8 pcf  
 Moisture: 37.5 %  
 Sample Diameter: 2.42 inches  
 Sample Length: 5.0 inches

**Unconfined Compressive Strength:** 1.8 ksf  
**Axial Strain at Failure:** 2.0 %  
**Strain Rate:** 1.04 %/min.

**SUMMARY OF UNCONFINED COMPRESSION (ASTM D2166) TEST RESULTS**

KAILUA ROAD INTERSECTION IMPROVEMENTS  
 VICINITY OF ULUOA STREET AND ULUMANU DRIVE  
 KAILUA, OAHU, HAWAII

## Responses to Request for Information (RFI'S / Questions)

Questions for solicitation: B24001205 61D-01-23 Kailua Rd. Int. Impvs, Vic. of  
Uluoa St. & Ulumanu Dr.  
01/02/2024

1. The "Contract Certification Date". Is that the date the contract is executed?

No, the "Contract Certification Date" is the date that the Department of Accounting and General Services (DAGS) certifies the contract funds.

2. The "Contract Certification Date". How are we notified of this date? Reason I am asking is the NTP is issued not more than 30 calendar days after the Contract Certification Date

No notification will be sent to the contractor. The "Contract Certification Date" is on the Contract Certification page which is immediately after the K sheets of the contract. This is in the contract that is distributed to the contractor.

3. Could the Project Completion Time of "180 Working Days from the Start Work Date" be extended by several months? Currently, Traffic Signal Mast Arm Poles, Traffic Cabinets, and Signal Framework have a Factory Lead Time of Approximately 25 Weeks. Also, this does not include the Equipment Submittal Review/Approval process.

The Project Completion Time will remain at 180 Working Days. Please bid accordingly.

4. The notice to bidders says the bid is due at 2:00 PM 1/18/24. However, the HIEPRO website says it is due Noon 1/18/24. Which is it Noon or 2:00 PM?

HIEPRO will be revised to 01/18/2024 2:00pm for Offer Due Date & Time.

5. Spec Section 672.02(D) and 627.04(B) Cellular Modem requires Modems to be provided with "static IP addresses". Static IP Addresses are provided by the Service Provider as part of Activation. Please verify that cost of FirstNet Activation is to be included.

FirstNet Activation costs will be paid by new item 627.1003 "Cellular Modem Data Service" force account. See Revised Special Provisions, Section 627.02(D), Section 627.04(B), and Section 627.06 for more information.

**6. Spec Section 672.02(D) and 627.04(B) Cellular Modem, Activation normally includes a Monthly Service Charge. Please state duration of FirstNet Wireless Service that should be included under this project. Example: Duration of project until project acceptance plus 1-Year?**

Once the signal is operational, monthly service charges will be paid by new item 627.1003 "Cellular Modem Data Service" force account when the signal is operational until completion of the project. See Revised Special Provisions Section 627.02(D), Section 627.04(B), and Section 627.06 for more information.

**7. Spec Section 672.02(D) calls for a "Single" Cellular Modem at the Ulua intersection with communication to interconnect and HDOT's Traffic Signal Central System. Spec Section 627.04 and 627.04(B) refers to "Cellular Modems" with communication to HDOT's CCTV Systems at the H-3 TOC. Please confirm if we are to provide one Cellular Modem at the Ulua intersection, or are we to provide a Cellular Modem at each of the intersections including Ulumanu and Kalaniana'ole Hwy./Ulukahi St. under this project.**

A total of three cellular modems will be installed for this project. See Revised Special Provisions, Section 627.02(D) and Section 627.04(B) for more information.

**8. Can we get profile drawings for the electric, traffic signal and street light ductlines.**

Profiles will not be provided. Please bid accordingly.

**9. Noticed there is no bid item for Traffic Signal Assembly (1-Way, 12-Inch, 1-3 Section Vertical with Type TP-2W Mounting {2 Heads}) = 1 Each.**

Bid Item has been added to the Proposal Schedule.